

GENERAL NOTES:

– DESIGN CODE: NATIONAL BUILDING CODE OF CANADA – LATEST ALBERTA BUILDING CODE – LATEST

DESIGN LOADS:

ENVIRONMENTAL LOADS

SHOW
S_w = 1.8 kPa (37.4 psf) + DRIFT
S_r = 0.1 kPa (2.1 psf)

RAIN (24HOUR)
1/50 = 95 mm (3.74") (SEE PONDING ON PLAN)

SEISMIC
S_a(0.2) = 0.12
S_a(0.5) = 0.06
S_a(1.0) = 0.02
S_a(2.0) = 0.01
P₀ = 0.06
SITE CLASS D

WIND
q(1/50) = 0.45 kPa (9.36 psf)
q(1/10) = 0.32 kPa (8.4 psf)

GENERAL

–THE CONTRACTOR SHALL OBTAIN WHATEVER FIELD DIMENSIONS ARE NECESSARY TO COMPLETE THE WORK CALLED FOR ON THE DRAWINGS. –DO NOT SCALE THE DRAWINGS.

–CHECK WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OPENINGS, INSERTS AND EMBEDMENTS REQUIRED IN CONCRETE.

–VERIFY ALL DIMENSIONS, ELEVATIONS, AND SCOPES OF WORK WITH THE DRAWINGS PRIOR TO COMMENCING CONSTRUCTION.

–IF ANY UNUSUAL STRUCTURAL CONDITIONS ARE CREATED OR OBSERVED DURING CONSTRUCTION, REPORT THEM IMMEDIATELY TO TRL & ASSOCIATES LTD.

–STRUCTURAL DRAWINGS SHOW THE COMPLETE STRUCTURE. THEY DO NOT SHOW COMPONENTS WHICH MAY BE NECESSARY FOR SAFETY DURING CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR SAFETY ON AND ABOUT THE WORK SITE DURING CONSTRUCTION.

–THE CONTRACTOR IS RESPONSIBLE FOR THE TEMPORARY RELOCATION OF PIPES, CONDUITS, PIPE HANGERS, ETC., THAT INTERFACE WITH CARRYING OUT THIS WORK.

–THESE NOTES AND DRAWINGS ARE TO READ IN CONJUNCTION WITH ALL OTHER RELATED DOCUMENTS.

–CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY BRACING, SHORING OR STRENGTHENING AS REQUIRED DURING CONSTRUCTION.

CONCRETE NOTES:

GENERAL NOTES:

–ALL CONCRETE, REINFORCEMENT, ACCESSORIES AND PROCEDURES SHALL MEET OR EXCEED THE APPLICABLE CSA STANDARD FOR THAT PRODUCT. USE ONLY PRODUCTS SUITABLE FOR THE INTENDED FINAL USE AND CONDITIONS PREVALENT DURING CONSTRUCTION. PROTECT ALL MATERIALS FROM THE WEATHER DURING STORAGE AND INSTALLATION.

–CEMENT: PORTLAND CEMENT AS REQUIRED CONFORMING TO CSA A301

–AGGREGATES: CLEAN, WELL-GRADED, UNCOATED SAND AND COARSE AGGREGATES FROM AN APPROVED SOURCE CONFORMING TO CAN/CSA-A23.1-M04.

–WATER: POTABLE FROM AN APPROVED MUNICIPAL SOURCE.

–ADMIXTURES: SHALL CONFORM WITH ASTM C260, 0494 OR C1017 AS APPLICABLE

–READY MIX CONCRETE: DESIGNED AND SUPPLIED BY THE SUPPLIER IN A QUALITY CONTROLLED PLANT CONFORMING TO CAN/CSA-A23.1-M04. UNLESS NOTED OTHERWISE, CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 25 MPa. SEE SCHEDULE THIS DRAWING FOR DETAILED REQUIREMENTS.

–CONCRETE EXPOSED TO FREEZE–THAW CONDITIONS SHALL MEET EXPOSURE CLASSIFICATION F–2, 25 MPa, MAXIMUM WATER/CEMENT RATIO 0.55, AIR CONTENT CATEGORY 2 AS SPECIFIED IN CAN/CSA-A23.1-M04, UNLESS NOTED OTHERWISE.

–CONCRETE EXPOSED TO DEICING CHEMICALS SHALL MEET EXPOSURE CLASSIFICATION C–2, 32 MPa, MAXIMUM WATER/CEMENT RATIO 0.45, AIR CONTENT CATEGORY 1 AS SPECIFIED IN CAN/CSA-A23.1-M04 UNLESS NOTED OTHERWISE.

–CONCRETE EXPOSED TO MULTIPLE EXPOSURE CONDITIONS SHALL MEET COMBINED EXPOSURE CLASSIFICATION REQUIREMENTS TO THE MOST SEVERE COMBINATION AS SPECIFIED IN CAN/CSA-A23.1-M04 UNLESS NOTED OTHERWISE.

–SLUMP SHALL BE WITHIN THE RANGE OF 50mm to 100mm (2" to 4"). GREATER SLUMPS SHALL NOT BE ACCEPTED UNLESS OTHERWISE SPECIFIED.

–PROVIDE AN APPROVED WATER REDUCING AGENT IN ALL CONCRETE MIX DESIGNS.

–USE OF FLYASH IS PERMITTED, WITH SUBSTITUTION TO LEVELS AS INDICATED IN THE SPECIFICATIONS. –CONTRACTOR TO USE APPROPRIATE MEASURES FOR CURING AND FINISHING

–PLACE CONCRETE AS A CONTINUOUS OPERATION STOPPING ONLY AT CONSTRUCTION JOINTS. CONSTRUCTION JOINTS SHALL BE ADEQUATELY DOWELED AND KEED. DETAILS AND LOCATIONS OF CONSTRUCTION JOINTS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER.

–ALL CONCRETE SHALL BE PLACED IN ITS FINAL POSITION WITHIN 2 HOURS OF ORIGINAL BATCHING.

–CONCRETE TESTING SHALL BE CARRIED OUT IN ACCORDANCE WITH CSA STANDARD CAN3-A23.2-M04 "METHODS OF TEST FOR CONCRETE" BY AN INDEPENDENT MATERIALS CONSULTANT, WITH REPORTS SUBMITTED TO THE STRUCTURAL ENGINEER.

–CURING PROCEDURES AND PROTECTION OF CONCRETE SHALL CONFORM TO THE REQUIREMENTS OF CSA STANDARD CAN/CSA-A23.1-M04. NEW CONCRETE SHALL NOT BE ALLOWED TO FREEZE UNDER ANY CIRCUMSTANCES. THE CONTRACTOR SHALL PAY THE COSTS RELATED TO DAMAGE BY UNDER STRENGTH OR IMPROPERLY CURED CONCRETE.

REINFORCEMENT NOTES:

–ALL REINFORCING BARS SHALL BE MANUFACTURED AND MEET THE REQUIREMENTS OF CSA STANDARD G 30.18–M09, BILLET

–STEEL BARS FOR CONCRETE REINFORCEMENT.

–ALL REINFORCING BARS SHALL BE GRADE 400 MPa(60 ksi).

–EPOXY COATED REINFORCEMENT, WHERE SPECIFIED, SHALL BE MANUFACTURED, FABRICATED, STORED, HANDLED, AND INSTALLED IN STRICT ACCORDANCE WITH THE CSA STANDARDS AND INDUSTRY PRACTICE.

–SPICES, BENDS, AND PLACEMENT SHALL CONFORM TO CAN/CSA-A23.1-M04 AND CAN3 A23.3-M04. REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH THE LATEST VERSION OF THE ACI DETAILING MANUAL. PROVIDE MATCHING CORNER BARS FOR ALL HORIZONTAL BARS AS DETAILED.

–ALL REINFORCING STEEL SHALL BE CHAIRED AND SECURELY TIED IN PLACE USING STANDARD TIES AND CHAIRS.

–ALL WELDED WIRE MESH SHALL BE MANUFACTURED AND MEET THE REQUIREMENTS OF CSA STANDARD G 30.5 WELDED STEEL WIRE FABRIC FOR CONCRETE REINFORCEMENT.

–ALL WELDED WIRE MESH (WWM) SHALL BE SUPPLIED IN FLAT SHEETS. ALL WWM SHALL BE CHAIRED IN PLACE TO THE REQUIRED COVER AS SPECIFIED.

PILE FOUNDATIONS

GENERAL:

–ALL PILES SHALL BE DESIGNED BY THE PILING CONTRACTOR AND HIS PROFESSIONAL ENGINEER TO CARRY THE SPECIFIED LOADS INDICATED ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL UTILIZE THE INFORMATION CONTAINED IN THE SOILS REPORT IF AVAILABLE. IF NO REPORT IS AVAILABLE, THE PILES SHALL BE DESIGNED BASED ON THE CONTRACTOR'S OWN INVESTIGATIONS AND EXPERIENCE IN THIS AREA. THE FINDINGS OF THE SOILS REPORT, CONTRACTOR INVESTIGATIONS, AND ASSUMPTIONS SHALL BE VERIFIED ON AN ONGOING BASIS DURING THE PILE INSTALLATION. THE PILE DESIGNS SHALL BE REVISED TO CARRY THE SPECIFIED LOADS IF CONDITIONS VARY.

–THE PILING CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW BEARING THE SEAL OF A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE PROVINCE OF ALBERTA. THE SHOP DRAWINGS SHALL INDICATE PILE CAPACITY AND REINFORCEMENT AS WELL AS PROPOSED LENGTHS AND CUT-OFFS. THE PILING CONTRACTOR SHALL PROVIDE A SEALED CERTIFICATE OF COMPLIANCE AFTER INSTALLATION.

–ALL CONCRETE SHALL FOLLOW THE CONCRETE SCHEDULE THIS DRAWING.

–ALL CONCRETE SHALL BE PLACED AS CLOSE TO ITS FINAL POSITION AS POSSIBLE TO PREVENT SEGREGATION OF THE MIX.

–SUPPLY AND PLACEMENT OF ALL TEMPORARY SHORING AND BRACING IS THE CONTRACTORS RESPONSIBILITY AND SHALL MEET ALL APPLICABLE STANDARDS AND LAWS.

–PILING CONTRACTOR TO DESIGN FOUNDATIONS AS A PROPRIETARY SYSTEM WITH ALLOWABLE SKIN FRICTION AND BEARING VALUES DETERMINED FROM A SOILS INVESTIGATION.

–ALL DISCREPANCIES IN DETAILS AND DIMENSIONS SHALL BE BROUGHT TO THE ATTENTION OF TRL & ASSOCIATES LTD. PRIOR TO COMMENCING RELATED WORK.

–THESE NOTES AND DRAWINGS ARE TO BE READ IN CONJUNCTION WITH ALL RELATED DOCUMENTS.

–REFER TO THE SOILS REPORT PREPARED BY HAGSTROM GEOTECHNICAL SERVICES LTD. EDMONTON, ALBERTA DATED APRIL 2012 FILE # H0805-168

GRADE BEAMS:

–ALL CONCRETE SHALL FOLLOW THE CONCRETE SCHEDULE THIS DRAWING.

–LAP TOP BARS 36 BAR DIAMETERS AT MID SPANS AND BOTTOM BARS 12 BAR DIAMETERS AT PILE/PIER SUPPORT LOCATIONS. WHERE SPLICES ARE LOCATED WITHIN TENSION ZONES OF THE CONCRETE, PROVIDE "CLASS B" LAP SPLICES IN ACCORDANCE WITH CAN/CSA-A23.1.

–UNLESS SHOWN OTHERWISE, FOUNDATIONS SHALL BE BACKFILLED EVENLY ON BOTH SIDES TO PREVENT MOVEMENT. BACKFILL HEIGHTS SHALL NOT VARY BY MORE THAN 300mm (12 INCHES) FROM ONE SIDE TO THE OTHER. EXERCISE EXTREME CAUTION DURING BACK FILL OPERATIONS TO PREVENT DAMAGE TO THE CONCRETE.

–DEGRADABLE "STYROFOAM" TYPE VOID FORM WITH CASTILATED (TOOTHED) CONFIGURATION SHALL BE USED BELOW ALL GRADE BEAMS SUBJECTED TO FROST ACTION. VOID FORM IS FORMWORK.

FLOOR SLAB SUPPORTED ON GRADE NOTES:

–REMOVE ALL TOP SOIL, ORGANICS, FROZEN SOIL, WET AND/OR WEAK SOILS. REFER TO THE SOILS REPORT FOR AVERAGE DEPTHS OF POOR SOIL. PROOF ROLL SUB-GRADE TO FURTHER DETECT SOFT AREAS. NATIVE, UNDISTURBED SOILS SHALL BE COMPACTED TO A UNIFORM DRY DENSITY OF 95% STANDARD PROCTOR MAXIMUM DRY DENSITY.

–BACKFILL AS REQUIRED BY CONDITIONS WITH 75mm (3 INCH MINUS) PIT RUN GRAVEL OR OTHER PREVIOUSLY APPROVED SOIL. FINAL 150mm (6 INCHES) SHALL BE 25 mm (1 INCH MINUS) WELL GRADED CRUSHER GRAVEL. MIX BACKFILL SHALL BE COMPACTED TO UNIFORM DRY DENSITY OF 98% STANDARD PROCTOR MAXIMUM DRY DENSITY. IN COMPACTED LAYERS NOT EXCEEDING 150mm (6 INCHES), BACKFILL DEPTHS EXCEEDING 1200mm (4 FEET) SHALL BE COMPACTED TO A UNIFORM DRY DENSITY OF 100% STANDARD PROCTOR MAXIMUM DRY DENSITY. IN COMPACTED LAYERS NOT EXCEEDING 150mm (6 INCHES).

–ALL OF THE ABOVE SHALL BE REVIEWED BY AN APPROVED SOILS TESTING FIRM INCLUDING ALL LIFTS OF BACKFILLING AT THE OWNERS COST.

–REINFORCEMENT SHALL BE CHAIRED OFF THE SUB GRADE PRIOR TO PLACING CONCRETE. PRE-MOISTEN THE GRAVEL PRIOR TO PLACING CONCRETE IF NO VAPOUR BARRIER IS REQUIRED.

–CONCRETE SHALL BE PLACED, SCREEDED AND FLOATED TO ENSURE A WELL COMPACTED, VOID FREE SLAB. THE FLOOR FINISH TOLERANCE SHALL BE CLASSIFIED AS "CONVENTIONAL" [WITHIN 12mm OF A 3 METER (0.5 INCH OF A 10 FOOT) STRAIGHT EDGE] IN ACCORDANCE WITH CAN/CSA-A23.1-M04.

–FINISH SHALL BE IN ACCORDANCE WITH CSA STANDARDS AND AS SPECIFIED ON THE DRAWINGS.

–FOR SLUMPS AND UNDER SLAB WEEPING TILE REQUIREMENTS REFER TO MECHANICAL DRAWINGS AND GEOTECHNICAL REPORT

FORMWORK AND FALSEWORK

– FABRICATE AND ERECT FORM WORK IN ACCORDANCE WITH CAN/CSA-S269.3 TO PRODUCE FINISHED CONCRETE CONFORMING TO SHAPE, DIMENSIONS, LOCATIONS AND LEVELS INDICATED WITHIN TOLERANCES REQUIRED BY CAN/CSA-A23.1.

– CLEAN FORMWORK IN ACCORDANCE WITH CAN/CSA-A23.1, BEFORE PLACING CONCRETE.

– PREPARE ENGINEERED SEALED SHOP DRAWINGS FOR ALL FORMWORK AND FALSEWORK. DRAWINGS TO BE SEALED BY A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE PROVINCE OF ALBERTA.

– UNLESS OTHERWISE NOTED LEAVE FORMWORK IN PLACE FOR THE FOLLOWING MINIMUM PERIODS OF TIME AFTER PLACING CONCRETE

– WALLS 7 DAYS
– COLUMNS 1 DAY
– STRUCTURAL BEAMS AND SLABS 7 DAYS OR THREE DAYS WHEN REPLACED BY ADEQUATE RESHORING
– FOOTINGS AND ABUTMENTS 3 DAYS

– REMOVE FORMWORK WHEN THE CONCRETE HAS ACHIEVED 80% OF ITS DESIGN STRENGTH OR MINIMUM PERIODS NOTED ABOVE, WHICHEVER COMES LATER AND REPLACE IMMEDIATELY WITH ADEQUATE RESHORING.

– RESHORING TO REMAIN IN PLACE UNTIL CONCRETE HAS ACHIEVED FULL 28 DAY DESIGN STRENGTH AS VERIFIED BY CONCRETE TEST IN ACCORDANCE WITH A23.1

– PROVIDE ALL NECESSARY RESHORING OF MEMBERS WHERE EARLY REMOVAL OF FORMS MAY BE REQUIRED OF WHERE MEMBERS MAY BE SUBJECTED TO ADDITIONAL LOADS DURING CONSTRUCTION AS REQUIRED.

– SPACE RESHORING IN EACH PRINCIPAL DIRECTION AT NOT MORE THAN 3000 mm APART.

– RE-USE FORMWORK AND FALSEWORK SUBJECT TO REQUIREMENTS OF CAN/CSA-A23.1.

STRUCTURAL STEEL NOTES:

GENERAL:

–ALL STRUCTURAL STEEL, MISCELLANEOUS STEEL, ACCESSORIES AND PROCEDURES SHALL MEET OR EXCEED THE APPLICABLE CSA STANDARD FOR THAT PRODUCT. USE ONLY PRODUCTS SUITABLE FOR THE INTENDED FINAL USE AND CONDITIONS PREVALENT DURING CONSTRUCTION. PROTECT ALL MATERIALS FROM THE WEATHER DURING STORAGE AND INSTALLATION.

–DESIGN, DETAIL, AND FABRICATE ALL CONNECTIONS IN A QUALITY CONTROLLED SHOP TO CSC HANDBOOK OF STEEL CONSTRUCTION. UNLESS OTHERWISE INDICATED ON THE DRAWINGS CONNECTIONS ARE TO BE DESIGNED FOR 50% OF THE FACTORED SHEAR CAPACITY OF THE MEMBER, WITH A MINIMUM CONNECTION OF TWO BOLTS. SHOP DRAWINGS BEARING THE STAMP OF A PROFESSIONAL ENGINEER LICENSED TO PRACTICE IN THE PROVINCE OF ALBERTA ARE TO BE SUBMITTED FOR THE DESIGN OF ALL CONNECTIONS.

–PAINTING: ONE COAT SHOP PRIMER TO DISC/CPMA STANDARD 1-73A-COLOR AS PER ARCH.

–STRUCTURAL STEEL: STEEL SECTIONS W.5 & WT CONFORMING TO CSA G40-20 AND CSA-G40.21, GRADE 350W OR ASTM A992 & A572 GRADE 50.

–STRUCTURAL STEEL: HOLLOW STRUCTURAL SECTIONS CONFORMING TO CAN3-G40.20-M81 AND CAN3-G40.21-M81, GRADE 350W.

–STRUCTURAL STEEL: STEEL PIPE SECTIONS CONFORMING TO ASTM A36

–STRUCTURAL STEEL PLATES, ROOFS, ANGLES & CHANNEL SHAPES SHALL CONFORM TO CSA G40.20-04 & CSA G40.21-04

–ANCHOR RODS: ANCHOR RODS, NUTS AND WASHERS CONFORMING TO ASTM F1554 (HOT DIPPED GALVANIZED WHERE SPECIFIED ON DRAWINGS).

–STRUCTURAL BOLTS: STRUCTURAL BOLTS, NUTS AND WASHERS CONFORMING TO ASTM A325.

–BOLLARDS: PIPE BOLLARDS FABRICATED FROM STANDARD WALL PIPE OR HSS QUALITY STEEL WITH A MINIMUM WALL THICKNESS OF 6.35 mm (0.25 INCH) (HOT DIPPED GALVANIZED WHERE SPECIFIED ON DRAWINGS).

–WELDING: WELDING, MATERIALS AND PROCEDURES CONFORMING TO CSA-W59-1982. ALL WELDING TO BE PERFORMED BY CERTIFIED WELDERS. A COPY OF CERTIFICATE SHALL BE FORWARDED TO TRL & ASSOCIATES LTD. AT THE START OF THE PROJECT WITH THE SHOP DRAWINGS AND AT THE START OF FIELD ERECTION FOR FIELD WELDING.

–GALVANIZING: HOT DIPPED GALVANIZING CONFORMING TO CSA-G164-M1981, 2 MINIMUM 600 g/m.

EXECUTION:

–FABRICATE AND ERECT STEEL IN ACCORDANCE WITH CSA S16.1.

–DO NOT FIELD CUT MEMBERS WITHOUT WRITTEN PERMISSION FROM TRL & ASSOCIATES LTD.

–REPAIR ALL DAMAGE TO GALVANIZED FINISHES USING GALVALLOY.

STRUCTURAL FIELD REVIEW:

–THE CONTRACTOR SHALL COOPERATE WITH ALL TESTING, INSPECTION AND QUALITY CONTROL PERSONNEL REQUIRED ON THE SITE AND WILL PROVIDE CASUAL LABOUR FORCES AS REQUIRED TO ASSIST IN ALL THE FIELD REVIEW PROCEDURES. THE CONTRACTOR SHALL GIVE REASONABLE NOTICE TO THESE AGENCIES PRIOR TO REQUIREING THEIR SERVICES.

–ALL REINFORCEMENT SHALL BE REVIEWED IN PLACE PRIOR TO PLACING THE CONCRETE BY TRL & ASSOCIATES LTD. ALL REINFORCEMENT SHALL BE IN PLACE AND SECURED AT THE TIME OF THE REVIEW. PROVIDE 24 HOURS NOTICE PRIOR TO POURS.

MATERIALS

–STEEL COATINGS TO A 591 --- STEEL SHEET, COLD-ROLLED, ELECTROLYTIC ZINC-COATED.

–STEEL TO CAN3-S136 AND SHALL BE IDENTIFIED AS TO SPECIFICATION, TYPE, GRADE AND MECHANICAL PROPERTIES.

OPEN WEB STEEL JOISTS AND STEEL DECKING:

–OWSI SUPPLIER SHALL DESIGN THE JOISTS USING THE SELF WEIGHT OF THE STRUCTURE PLUS THE LIVE LOADS INDICATED PLUS MECH. UNIT LOADS INDICATED ON THE DWGS. THE NOMINAL SPACING AND DEPTH INDICATED ON THE DRAWINGS SHALL BE MAINTAINED. INDUSTRY STANDARD WEB CONFIGURATIONS SHALL BE USED TO ALLOW DUCTWORK TO PASS.

–ROOF JOISTS SHALL BE DESIGNED FOR SECOND ORDER DEFLECTIONS DUE TO ROOF PONDING

–FLOOR JOISTS SHALL BE DESIGNED WITH A MAXIMUM LIVE LOAD DEFLECTION OF L/480. CAMBER JOISTS FOR FULL DEAD LOAD

–ROOF JOISTS SHALL BE DESIGNED WITH A MAX. LIVE LOAD DEFLECTION OF L/360. CAMBER JOISTS FOR 50% LIVE LOAD AND 100% OF DEAD LOAD

–ALL JOISTS MARKED TJ (TIED JOIST) SHALL INCLUDE AN EXTENDED BOTTOM CHORD AND A CONNECTION TO THE ADJACENT COLUMN OR WALL.

–THE JOIST SUPPLIER SHALL SUBMIT ALBERTA ENGINEER SEALED SHOP DRAWINGS PRIOR TO FABRICATION.

–SUPPLY AND INSTALL AN L 75 x 75 x 6 (3" x 3" x 1/4") FRAME FOR ALL OPENINGS IN DECKING.

–SUPPLY AND INSTALL A C150 x 12 (C6 x 8.2) FRAME AT ALL MECHANICAL UNITS THAT ARE SUPPORTED BY OR HUNG FROM THE DECK OR JOISTS. REFER TO FRAMING ON THIS DRAWING.

–PAINTING: ONE COAT SHOP PRIMER TO DISC/CPMA STANDARD 1-73A-COLOR AS PER ARCH.

–ALL DECK MATERIAL AND INSTALLATION SHALL CONFORM TO THE CANADIAN SHEET METAL BUILDING INSTITUTE CODE OF PRACTICE, INCLUDING THE USE OF WELDERS CERTIFIED FOR THE TYPE OF WORK.

–ALL DECKING SHALL BE WELDED TO THE STRUCTURAL STEEL AT A MINIMUM OF 300 mm(12") CENTRES AND BUTION PUNCHED AT 450 mm (18") CENTRES, EXCEPT AS SHOWN ON THE DRAWINGS.

–UNLESS OTHERWISE SPECIFIED, ALL DECKING SHALL BE: ROOF 38 mm x 0.76 mm (1.5" x 22 GAUGE) HD 938 AS MANUFACTURED BY VIC WEST OR EQUIVALENT, WITH A ZF075 (WIPE COAT) ZINC COATING. FLOOR 38 mm x 0.76mm (1.5" x 22 GAUGE) HB938 AS MANUFACTURED BY VIC WEST OR EQUIVALENT WITH A ZF075 (WIPE COAT) ZINC COATING.

EXECUTION:

–FABRICATE AND ERECT STEEL IN ACCORDANCE WITH CSA S16.1.

–DO NOT FIELD CUT MEMBERS WITHOUT WRITTEN PERMISSION FROM TRL & ASSOCIATES LTD.

–REPAIR ALL DAMAGE TO GALVANIZED FINISHES USING GALVALLOY.

STEEL STUDS:

GENERAL:

–STEEL STUD STEEL FRAMING INCLUDES WIND BEARING STUDS, AXIAL LOAD BEARING STUDS, FLOOR JOISTS, CEILING JOISTS, ROOF JOISTS, BULKHEADS AND ROOF RAFTERS.

–UNLESS NOTED OTHERWISE ON DRAWINGS STUD SUPPLIER TO PROVIDE MIN. 16mm DEFLECTION TRACK BELOW ALL STRUCTURE FOR PARTITION WALLS AND WIND BEARING STUDS. –IN ACCORDANCE WITH CAN3-S136

–CONFORM TO THE REQUIREMENTS OF SPECIFIED FIRE RATED ASSEMBLIES.

–DESIGN BRIDGING TO PREVENT MEMBER ROTATION AND MEMBER TRANSULATION PERPENDICULAR TO THE MINOR AXIS. PROVIDE FOR SECONDARY STRESS EFFECTS DUE TO TORSION BETWEEN LINES OF BRIDGING.

–MAXIMUM DEFLECTIONS UNDER SPECIFIED LOADS SHALL CONFORM TO THE FOLLOWING:

1. LIVE LOAD DEFLECTIONS OF WALL STUDS SUPPORTING MATERIALS SUSCEPTIBLE TO CRACKING (EG. MASONRY VENEER L/720, WALL STUDS SUPPORTING ALL OTHER MATERIALS L/360 (EG. METAL CLADDING, MANUFACTURED VENEERS).

2. FLOOR JOISTS L/360.

3. ROOF JOISTS AND RAFTERS L/360.

4. BUILDING SWAY DUE TO ALL EFFECTS 1/400 OF BUILDING HEIGHT OR 1/500 OF STOREY HEIGHT.

–THE SPACING OF MEMBERS SHALL NOT EXCEED THE FOLLOWING:

WALL STUDS 400mm (16") O/C
FLOOR JOISTS 400mm (16") O/C
CEILING JOISTS 600mm (24") O/C
ROOF JOISTS 600mm (24") O/C
ROOF RAFTERS 600mm (24") O/C

–WIND BEARING METAL STUDS.

–WIND BEARING METAL STUDS SHALL CONFORM TO THE MINIMUM SIZES AND SPACINGS DEFINED IN THE SCHEDULE BELOW, UNLESS NOTED OTHERWISE.

(A) SUPPORTING MATERIALS OTHER THAN MASONRY SPAN (ft)	SIZE	SPACING	LIVE LOAD DEFLECTION
0-5750	800S162-43	400 o.c.	L/360
5750-6200	800S162-43	300 o.c.	L/360

–CONNECTIONS BETWEEN LIGHTWEIGHT STEEL FRAMING MEMBERS SHALL BE BY BOLTS, WELDING OR SHEET METAL SCREWS. RESISTANCES FOR SHEET METAL SCREWS SHALL BE BASED ON THE MANUFACTURER'S LOWER BOND TEST VALUES MULTIPLIED BY THE APPROPRIATE RESISTANCE FACTOR, AS GIVEN IN CAN3-S136 M.

–SUBMIT SHOP DRAWINGS BEARING THE STAMP AND SIGNATURE OF A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF ALBERTA. INCLUDE ALL NECESSARY SHOP DETAILS AND ERECTION DIAGRAMS. INDICATE MEMBER SIZES, LOCATIONS, THICKNESS EXCLUSIVE OF COATING, COATINGS AND MATERIALS. INCLUDE CONNECTION DETAILS FOR ATTACHING FRAMING TO ITSELF AND FOR ATTACHMENT TO THE STRUCTURE. SHOW SPlice DETAILS WHERE PERMITTED. INDICATE DESIGN LOADS.

EXECUTION

–WELDS SHALL CONFORM TO CSA W59 AND/OR AWS/AWS D1.3, WHICHEVER IS APPLICABLE. TOUCH-UP WELDS WITH ZINC RICH PAINT.

–SCREWS – PENETRATION BEYOND JOINED MATERIALS SHALL BE NOT LESS THAN 3 EXPOSED THREADS.

–LIGHTWEIGHT STEEL FRAMING SHALL BE ERECTED TRUE AND PLUMB WITHIN THE SPECIFIED TOLERANCES.

–ERECTION TOLERANCES IN ACCORDANCE WITH CESSB 50M.

–CUTTING OF MEMBERS MAY BE BY SAW OR SHEAR. TORCH CUTTING IS NOT PERMITTED.

–SPlicing OF AXIAL LOAD BEARING MEMBERS IS NOT PERMITTED.

HOLLOW CORE PRECAST/PRESTRESSED CONCRETE

1.0 GENERAL

1.1 DESCRIPTION
1 THE GENERAL CONDITIONS OF THE CONTRACT AND SUPPLEMENTARY GENERAL CONDITIONS APPLY TO THIS DIVISION, EXCEPT AS QUALIFIED HEREIN AND/OR EXCLUDED.
2 REFER TO THE DRAWINGS AND SPECIFICATIONS.

1.2 RELATED WORK

1 CAST-IN-PLACE CONCRETE: SECTION 03300
(1) DRYPACKING OF GAP BETWEEN PRECAST/PRESTRESSED SLABS AT ALL LOCATIONS WHERE LOAD BEARING WALLS ARE PARALLEL TO LENGTH OF SLAB.
(2) PERIMETER CAULKING.
(3) ELECTRICAL HOLES.

(4) CONCRETE TOPPING (MINIMUM 37 MM (1 1/2"))

1.3 REFERENCE STANDARDS

1 DO PRECAST/PRESTRESSED CONCRETE WORK IN ACCORDANCE WITH CSA A23.4 AND CSA A23.3.

2 DO WELDING IN ACCORDANCE WITH CSA W59 FOR WELDING TO STEEL STRUCTURES AND CSA W186 FOR WELDING REINFORCEMENT.

3 QUALIFICATIONS OF MANUFACTURER

1 FABRICATE PRECAST/PRESTRESSED CONCRETE ELEMENTS CERTIFIED BY THE CANADIAN STANDARD ASSOCIATION IN THE APPROPRIATE CATEGORY(IES) ACCORDING TO CSA STANDARD A23.4-00 "PRECAST CONCRETE – MATERIALS AND CONSTRUCTION". THE PRECAST CONCRETE MANUFACTURER SHALL BE CERTIFIED IN ACCORDANCE WITH THE CSA CERTIFICATION PROGRAM FOR STRUCTURAL PRECAST/PRESTRESSED CONCRETE PRIOR TO SUBMITTING A TENDER AND MUST SPECIFICALLY VERIFY AS PART OF HIS TENDER THAT HE IS CURRENTLY CERTIFIED IN THE APPROPRIATE CATEGORY(IES):

(A) PRECAST CONCRETE PRODUCTS – ARCHITECTURAL
(i) NON-PRESTRESSED OR (ii) PRESTRESSED
(B) PRECAST CONCRETE PRODUCTS – STRUCTURAL
(i) NON-PRESTRESSED OR (ii) PRESTRESSED

(C) PRECAST CONCRETE PRODUCTS – SPECIALTY
(i) NON-PRESTRESSED OR (ii) PRESTRESSED

ONLY PRECAST CONCRETE ELEMENTS FABRICATED BY CERTIFIED MANUFACTURERS ARE SUBJECT TO THE OWNER.

CERTIFICATION MUST BE MAINTAINED FOR THE DURATION OF THE FABRICATION AND ERECTION FOR THE PROJECT. FABRICATE PRECAST CONCRETE ELEMENTS IN ACCORDANCE WITH ALBERTA BUILDING CODE REQUIREMENTS.

2 THE PRECAST CONCRETE MANUFACTURER SHALL BE A MEMBER IN GOOD STANDING WITH THE CANADIAN PRECAST/PRESTRESSED CONCRETE INSTITUTE (CPCI) AND HAVE A PROVEN RECORD AND SATISFACTORY EXPERIENCE IN THE DESIGN, MANUFACTURE AND ERECTION OF PRECAST CONCRETE FACING UNITS OF THE TYPE SPECIFIED. THE COMPANY SHALL HAVE ADEQUATE FINANCING, EQUIPMENT, PLANT AND SKILLED PERSONNEL TO DETAIL, FABRICATE AND ERECT THE WORK OF THIS SECTION AS REQUIRED BY THE SPECIFICATION AND DRAWINGS. THE SIZE OF THE PLANT SHALL BE ADEQUATE TO MAINTAIN THE REQUIRED DELIVERY SCHEDULE.

1.5 DESIGN CRITERIA

1 DESIGN PRECAST/PRESTRESSED CONCRETE UNITS TO CSA A23.3 AND TO CARRY HANDLING STRESSES.

2 DESIGN LOADS IN ACCORDANCE WITH APPLICABLE CODES FOR USE AND OCCUPANCY, WIND, TEMPERATURE, AND EARTHQUAKE.

3 CONSIDER VIBRATION CHARACTERISTICS IN ACCORDANCE WITH NBC.

4 DESIGN PRESTRESSED UNITS TO MEET ONE (1) OR TWO (2) HOUR FIRE RESISTANT RATING

1.6 SOURCE QUALITY CONTROL

1 UPON REQUEST, PROVIDE ENGINEER WITH CERTIFIED COPIES OF QUALITY CONTROL TESTS AND INSPECTION RELATED TO PROJECT AS SPECIFIED IN CSA A23.4-00 CSA G279.

2 INSPECTION OF PRESTRESSED CONCRETE TENDONS IS REQUIRED IN ACCORDANCE WITH CSA G279.

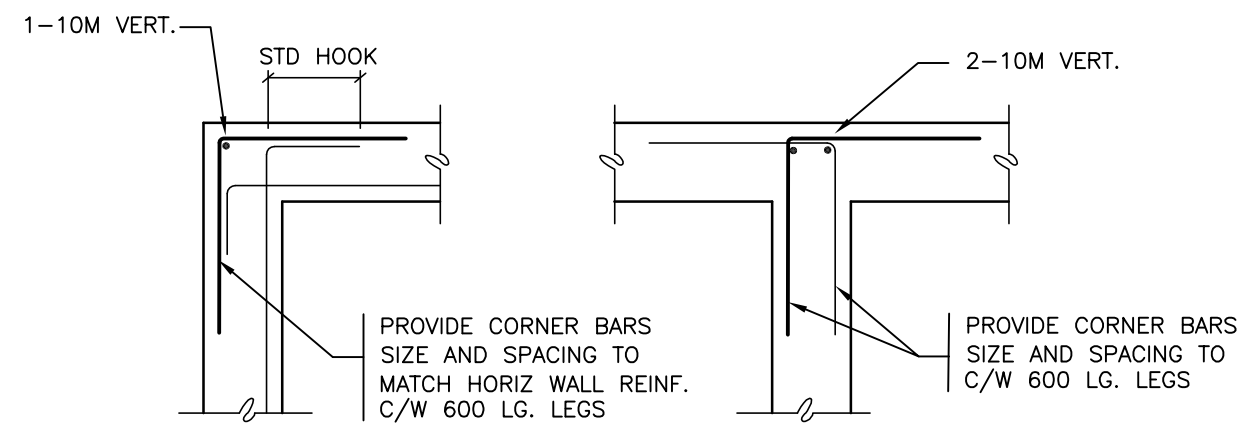
3 UPON REQUEST, PROVIDE AN ENGINEER WITH CERTIFIED COPY OF MILL TEST REPORT OF REINFORCING STEEL SUPPLIED, SHOWING PHYSICAL AND CHEMICAL ANALYSIS.

1.7 SHOP DRAWINGS

1 SUBMIT SHOP DRAWINGS IN ACCORDANCE WITH SECTION 01340 SHOP DRAWINGS, PRODUCT DATA.

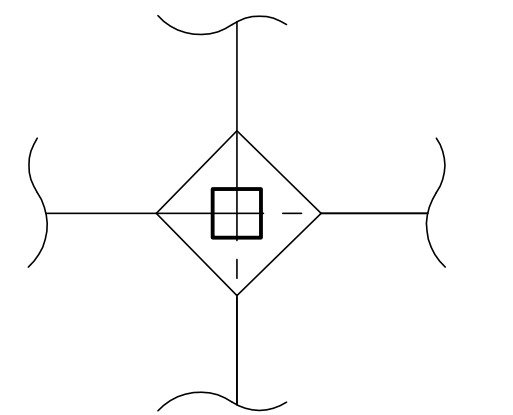
2 SUBMIT SHOP DRAWINGS IN ACCORDANCE WITH CSA A23.4 AND CSA A23.3. UPON REQUEST, THE FOLLOWING ITEMS SHALL BE PROVIDED:

1 DESIGN CALCULATIONS FOR ITEMS DESIGNED BY THE MANUFACTURER
2 ESTIMATED CAMBER
3 FINISHING SCHEDULES
4 METHODS OF

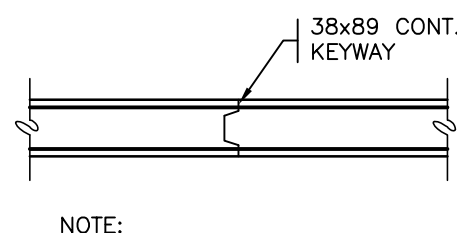


TYPICAL CORNER BAR DETAILS

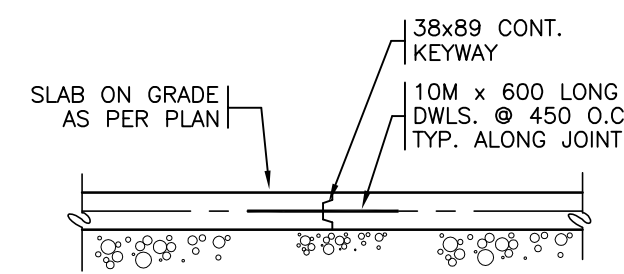
NOTES:
-TYPICAL ALL CORNERS AND WALL INTERSECTIONS



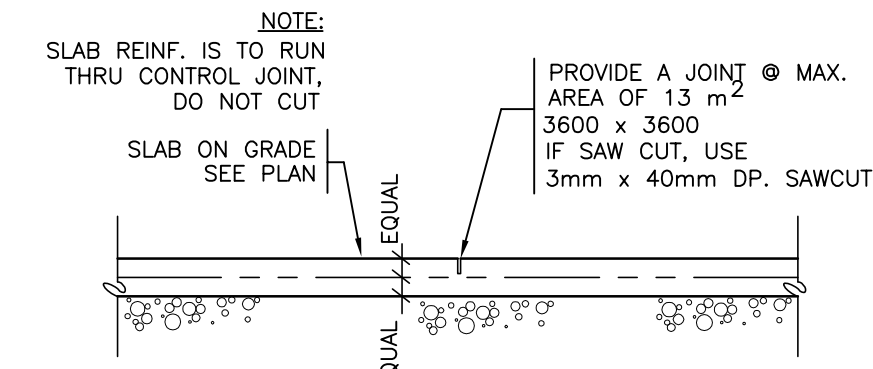
TYP. PLAN FOR SLAB CUT AROUND ALL INT. COLUMNS



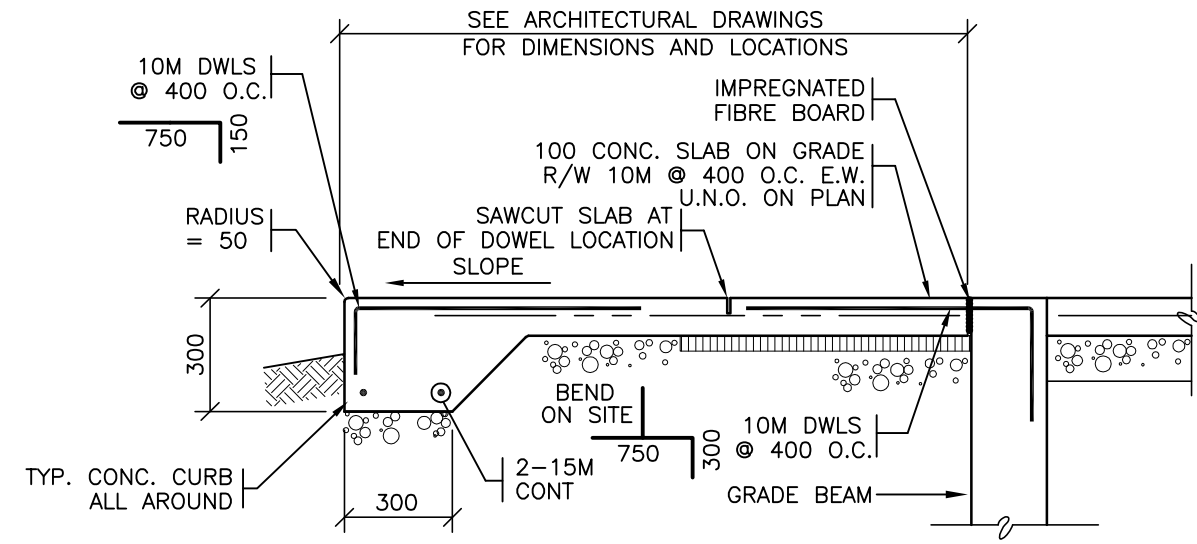
GRADE BEAM CONSTRUCTION JOINT
TYPICAL AS REQUIRED



TYPICAL SLAB ON GRADE CONSTRUCTION JOINT
NOTES:
-SEE ALSO NOTES THIS DRAWING

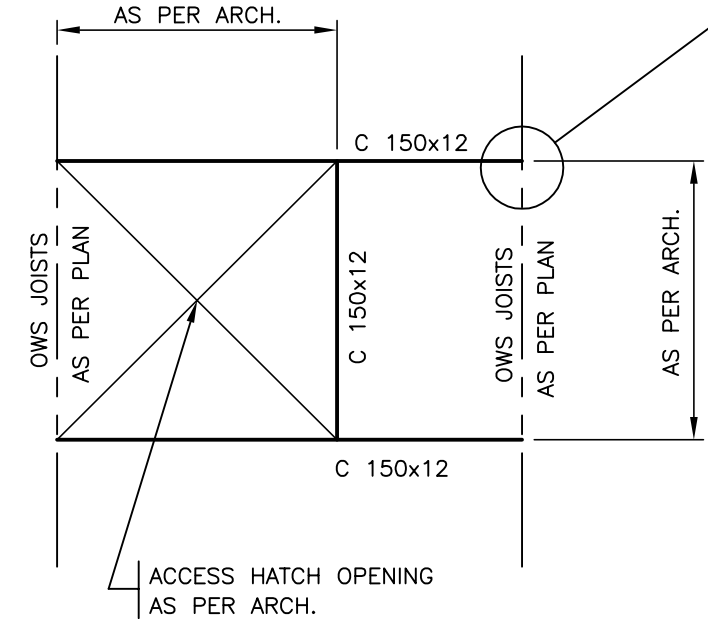


TYPICAL SLAB ON GRADE CRACK CONTROL JOINT

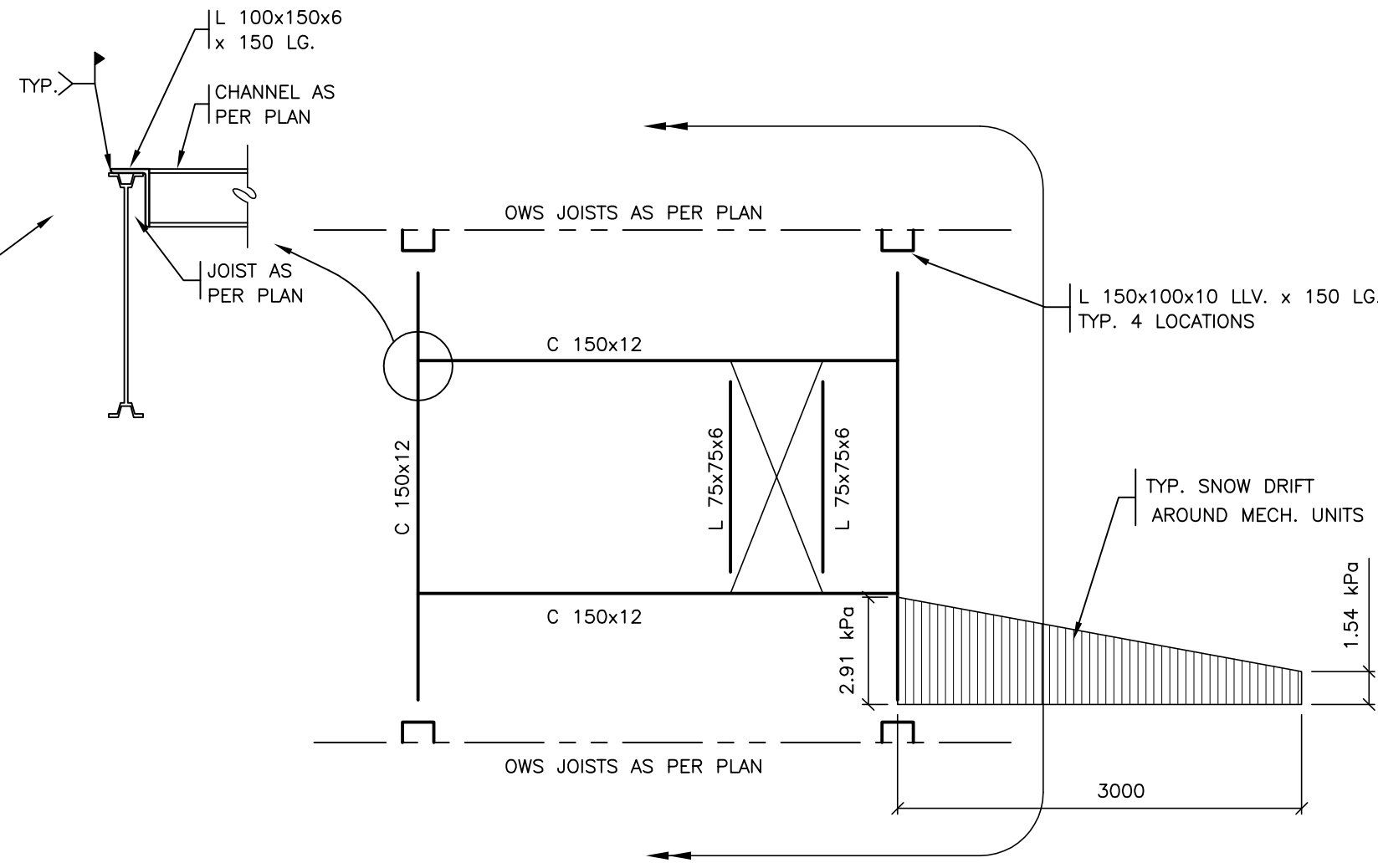


SIDEWALK SLAB

REFER TO ARCH. FOR EXACT SIZE & LOCATION
NOTE: PROVIDE CONT. 50mm RIGID INSULATION FOR 1200mm AWAY FROM FOUNDATION WALL FOR ALL SIDEWALKS

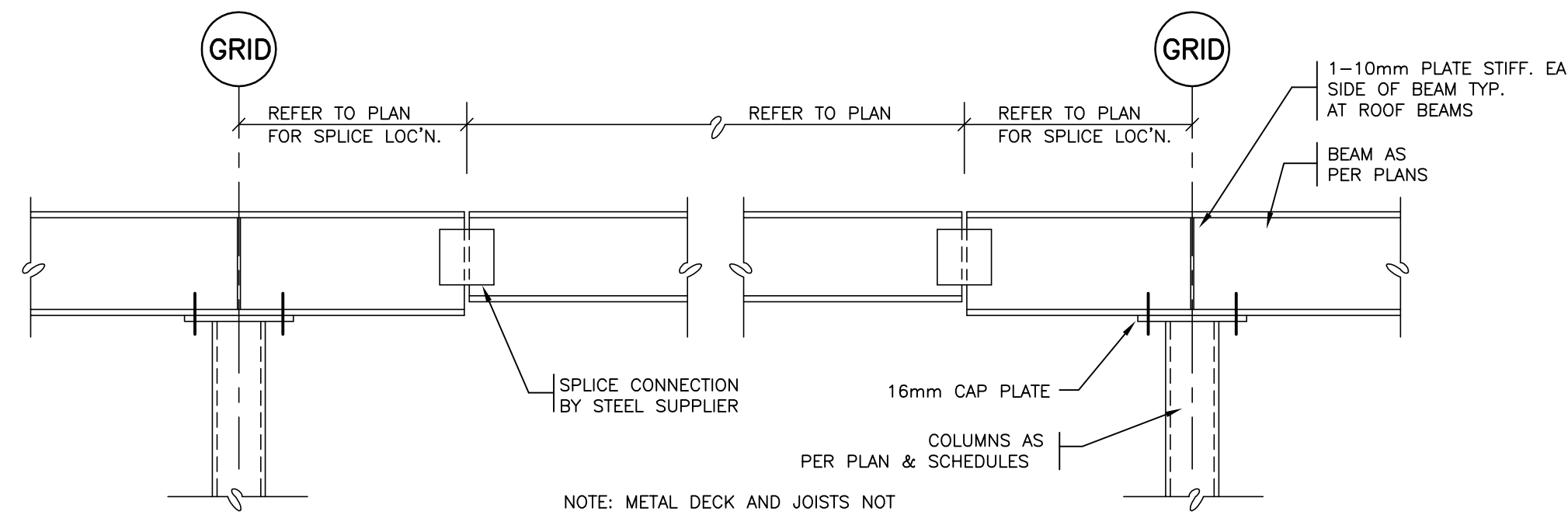


TYP. ROOF ACCESS HATCH FRAME
-REFER TO ARCH. FOR EXACT SIZE & LOCATION



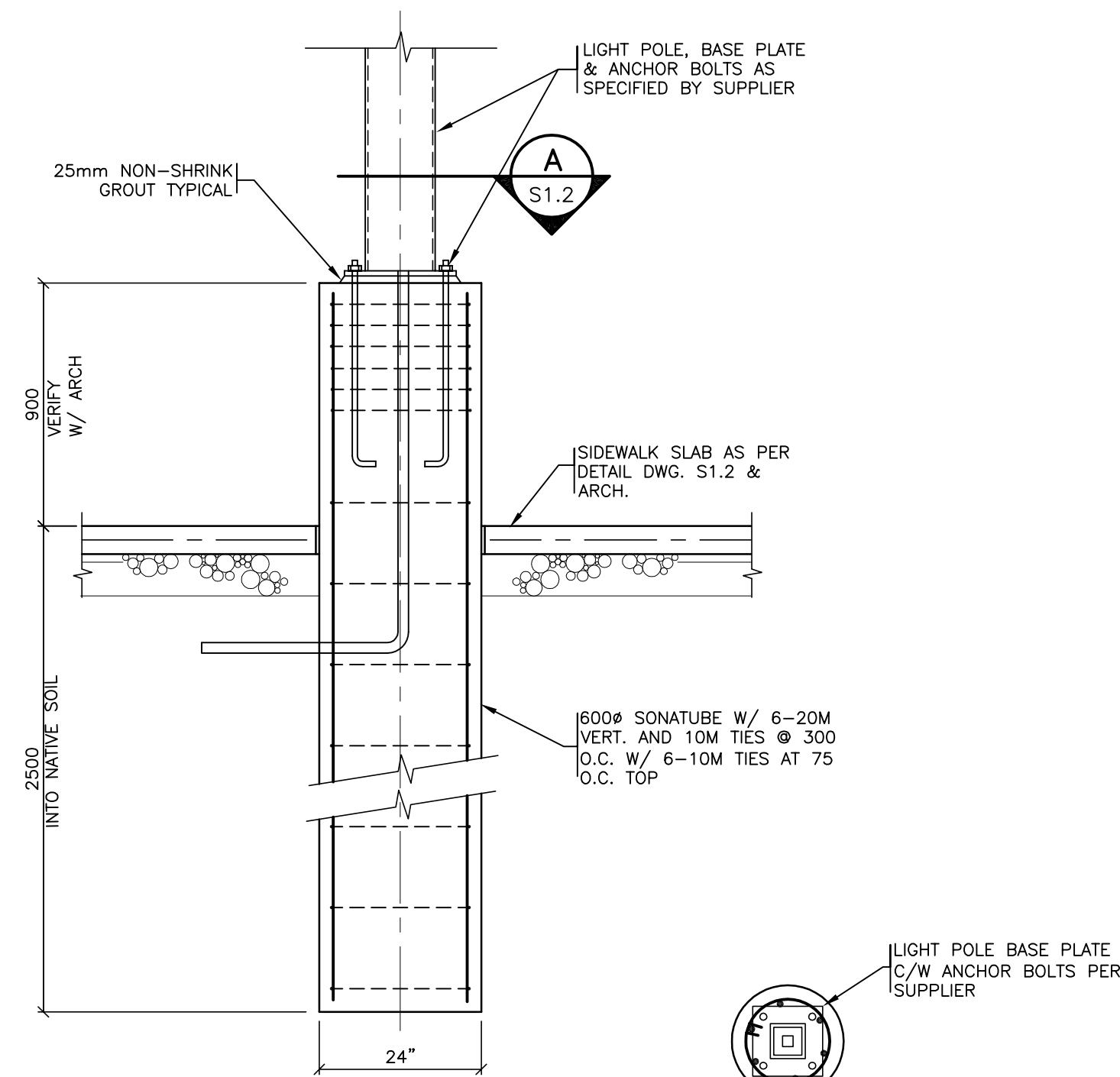
TYP. MECHANICAL SUPPORT FRAME

-REFER TO MECH. FOR SIZE, WEIGHT & LOCATION OF EQUIPMENT AND OPENINGS.



TYPICAL CANTILEVERED SUSPENDED SPAN BEAM DETAIL

NOTE:
REFER TO PLAN FOR COLUMN & BEAM TIE JOIST LOCATIONS



1 LIGHT POLE DETAIL
SCALE NTS

A PLAN
SCALE NTS



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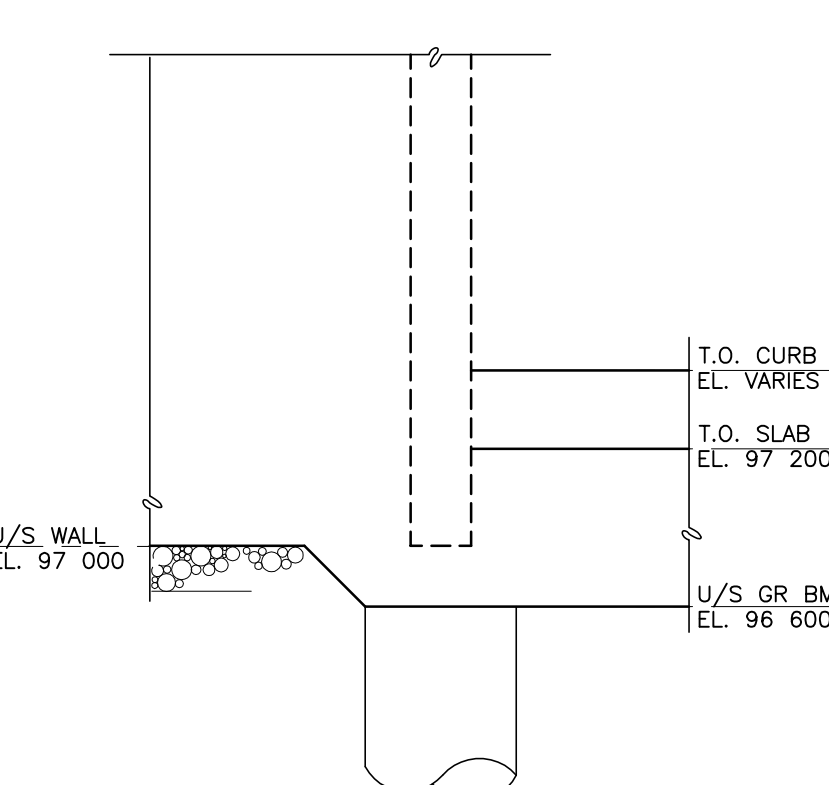
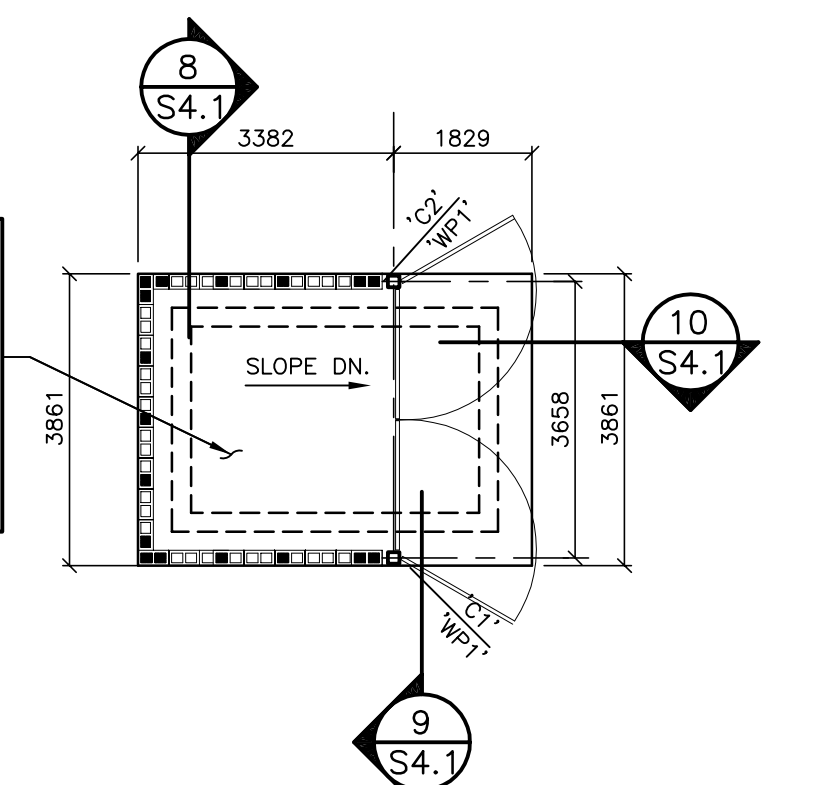
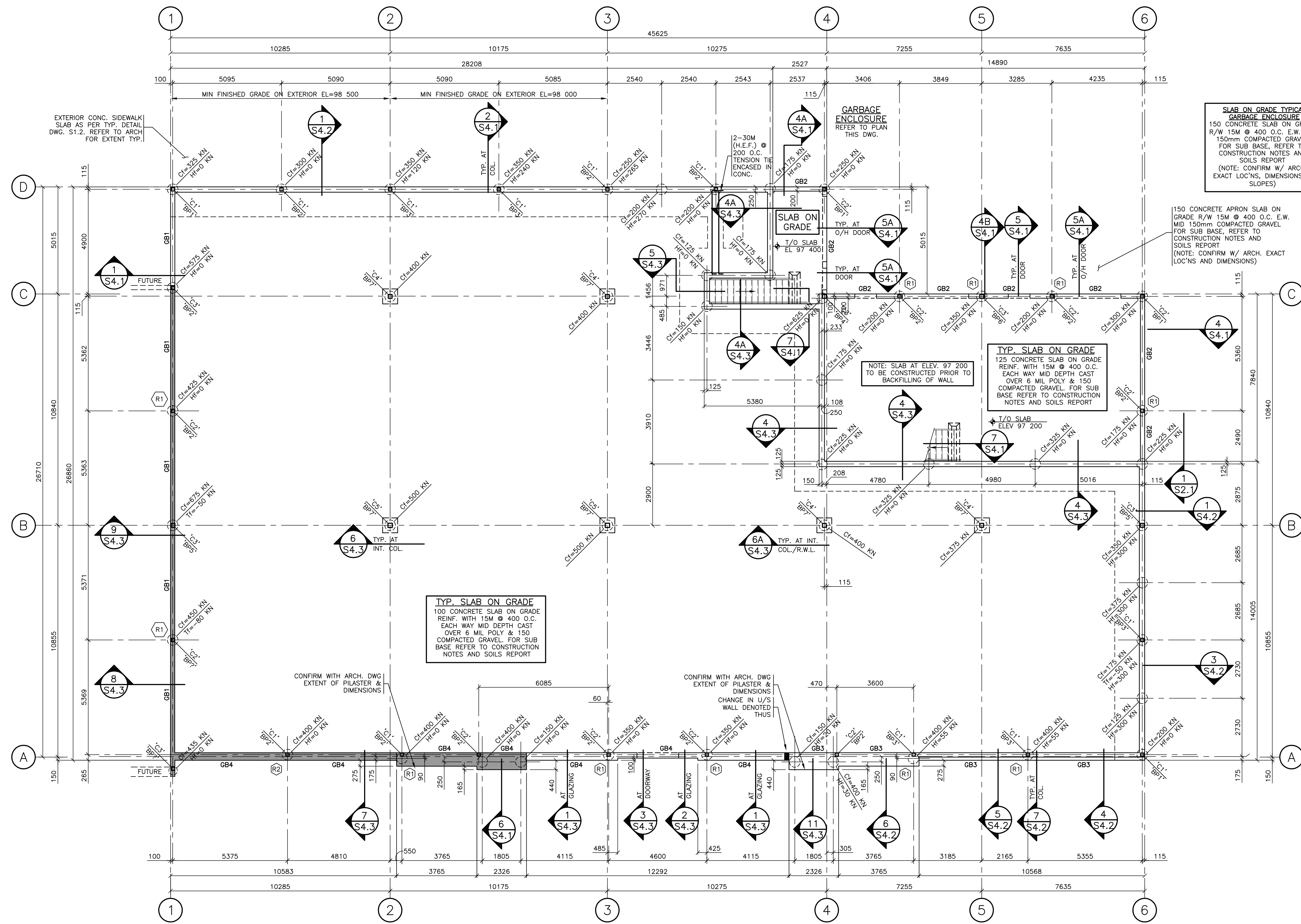
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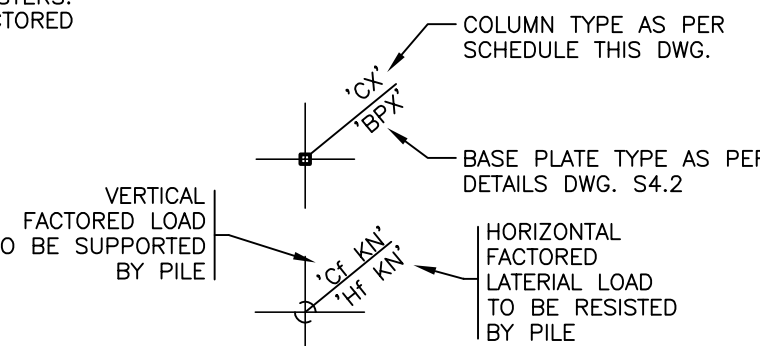
VISIONS
TYPICAL DETAILS

S1.2
2012-092



MAIN FLOOR & FOUNDATION PLAN

- SCALE 1:100
- NOTES:**
- TOP OF MAIN FLOOR SLAB ELEV. 100 000 U.N.O. SEE PLAN FOR SLOPE TO DRAIN COORDINATE WITH ARCHITECTURAL
 - 'GBY' ON PLAN DENOTES GRADE BEAM TYPE. REFER TO GRADE BEAM SCHEDULE ON THIS DRAWING
 - 'CX' ON PLAN DENOTES STEEL COLUMN TYPE. REFER TO STEEL COLUMN SCHEDULE ON THIS DRAWING
 - 'BPX' ON PLAN DENOTES BASE PLATE TYPE. REFER TO DRAWING S4.2
 - 'WPX' ON PLAN DENOTES WELD PLATE TYPE. REFER TO DRAWING S4.2
 - PILES ARE CENTRED BELOW GRADE BEAM TYPICAL UNLESS NOTED OTHERWISE. SEE PILEMASTER PLANS DWG S4.4 FOR PILE LOCATIONS AT PILEMASTERS.
 - ALL PILE LOADS ARE FACTORED



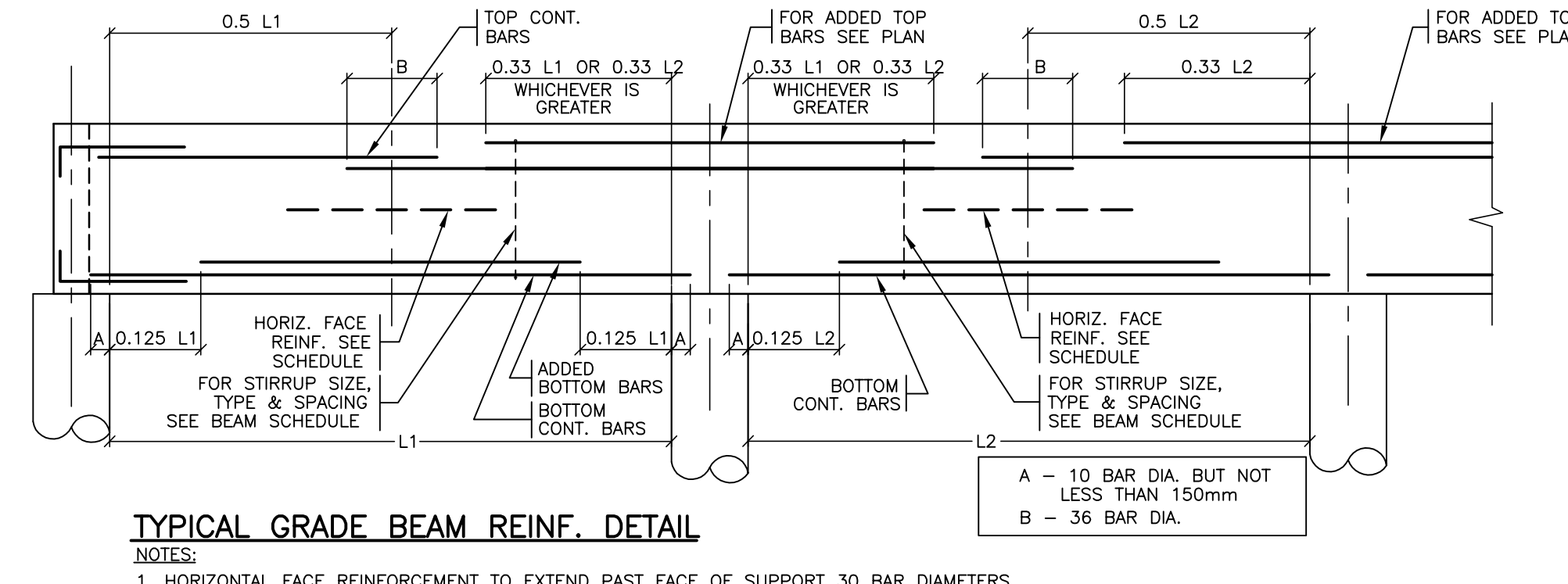
MARK	WIDTH	DEPTH	BOTTOM BARS		TOP BARS		STIRRUPS			ADDITIONAL REINFORCING	REMARKS
			CONT.	ADDED	CONT.	ADDED	SIZE	TYPE	SPACING		
'GB1'	200	1200	2-20M	-	2-20M	SEE PLAN	10M	□	300	ADD 15M @ 300 O.C. HORIZ. E.F.	CORNERS BARS AS PER DWG. S1.1
'GB2'	200	600	2-20M	-	2-15M	SEE PLAN	10M	□	250		CORNERS BARS AS PER DWG. S1.1
'GB3'	350	900	2-20M	-	2-20M	SEE PLAN	15M	□	250		CORNERS BARS AS PER DWG. S1.1
'GB4'	350 250	600	2-20M	-	2-20M	SEE PLAN	15M	□	400	ADD 15M @ 300 O.C. HORIZ. E.F.	CORNERS BARS AS PER DWG. S1.1

NOTES:
 REFER TO TYPICAL CORNER BAR DETAILS DWG. S1.1

(R1) - ADD 1-15M TOP ADD REINF. OVER SUPPORT (R3) - ADD 1-25M TOP ADD REINF. OVER SUPPORT
 (R2) - ADD 1-20M TOP ADD REINF. OVER SUPPORT (R4) - ADD 1-30M TOP ADD REINF. OVER SUPPORT

MARK	DESCRIPTION
'C1'	HSS 152 x 152 x 4.8
'C2'	HSS 152 x 152 x 6.4
'C3'	HSS 178 x 178 x 6.4
'C4'	HSS 203 x 203 x 6.4
'C5'	HSS 203 x 203 x 8.0

-ALL COLUMNS TO BE CLASS 'C' UNLESS NOTED OTHERWISE
 -ALL COLUMNS C/W 20mm BASE PLATE W/ A307 BOLTS & 16mm CAP PLATE TYPICAL U.N.O.



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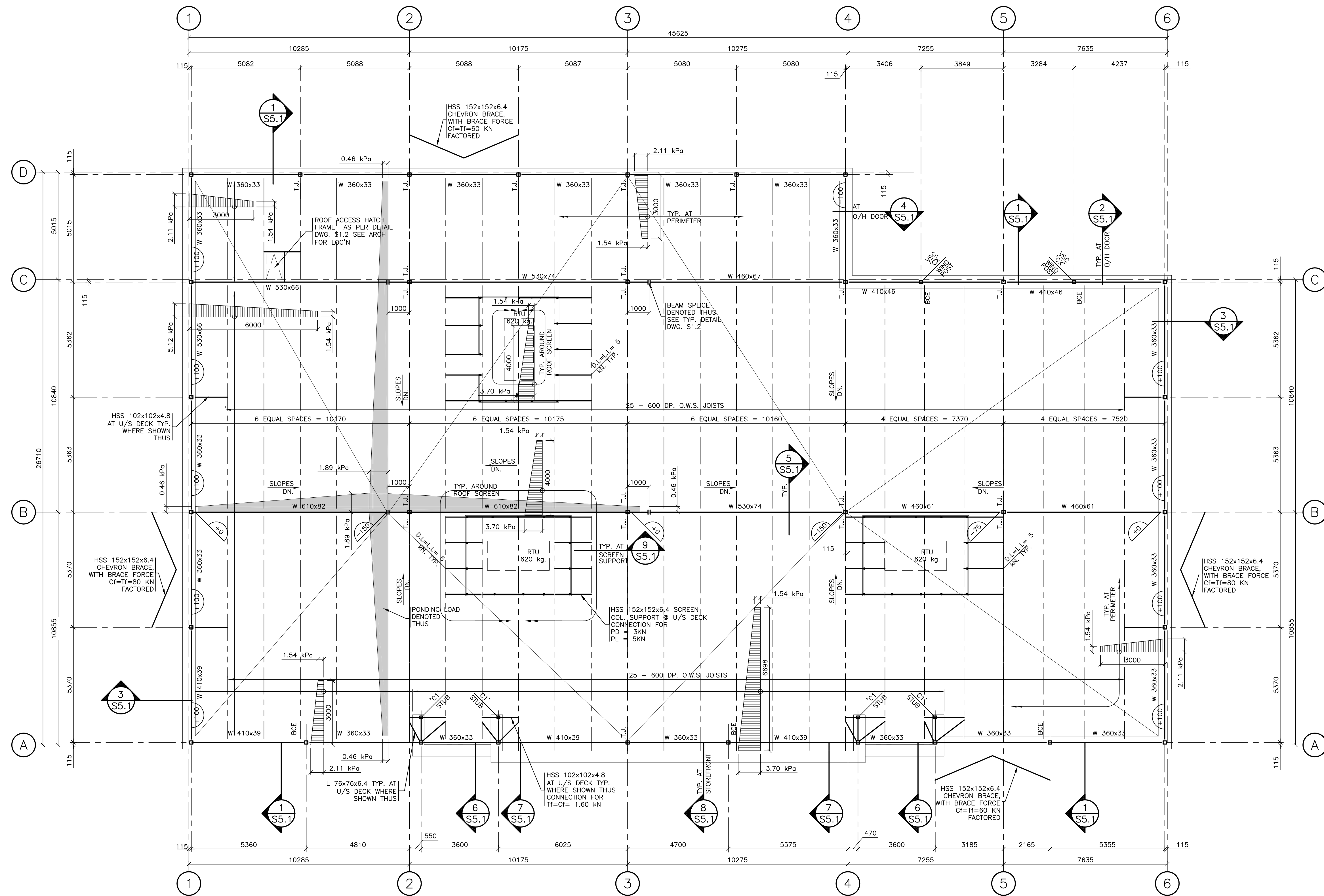
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VISIONS MAIN FLOOR & FOUNDATION PLAN

S2.1



ROOF FRAMING PLAN
SCALE = 1 : 100

- NOTES:**
- U/S ROOF DECK ELEV. 105 700
 - ROOF DECK TO BE 38mm x 0.76 (22 ga.) STEEL ROOF DECK
 - TOP OF ROOF STRUCTURAL STEEL ELEVATION 105 600 TYPICAL UNLESS NOTED THUS ON PLAN
 - REFER TO MECH. FOR EXACT SIZE, WEIGHT & LOCATION OF ALL ROOF DEAD LOAD = 1.35 kPa + MECHANICAL + PAVERS UNITS & OPENINGS. REFER TO DWG. S1.2 FOR FRAMING REQUIREMENTS NOT SHOWN ON PLAN.
 - ALL LOADS SPECIFIED ON THE DRAWINGS ARE UNFACTORED
 - T.J. ON PLAN DENOTES THE JOIST
 - B.C.E. ON PLAN DENOTES BOTTOM CHORD EXTENSION
 - FOR GENERAL NOTES & TYPICAL DETAILS REFER TO DWG. S1.1 & S1.2
- STEEL DECK WELDING REQUIREMENTS:**
- PUDDLE WELD @ 300 O.C. U.N.O.
 - BUTTON PUNCH @ 450 O.C.
 - LONGITUDINAL WELDS @ 900 O.C. MAX.
- ROOF DESIGN LOADS:**
- LIVE LOAD = 1.54 kPa + SNOWDRIFT OR PONDING WHICHEVER PROVIDES THE WORST EFFECT



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VISIONS
ROOF FRAMING
PLAN

S3.1
2012-092



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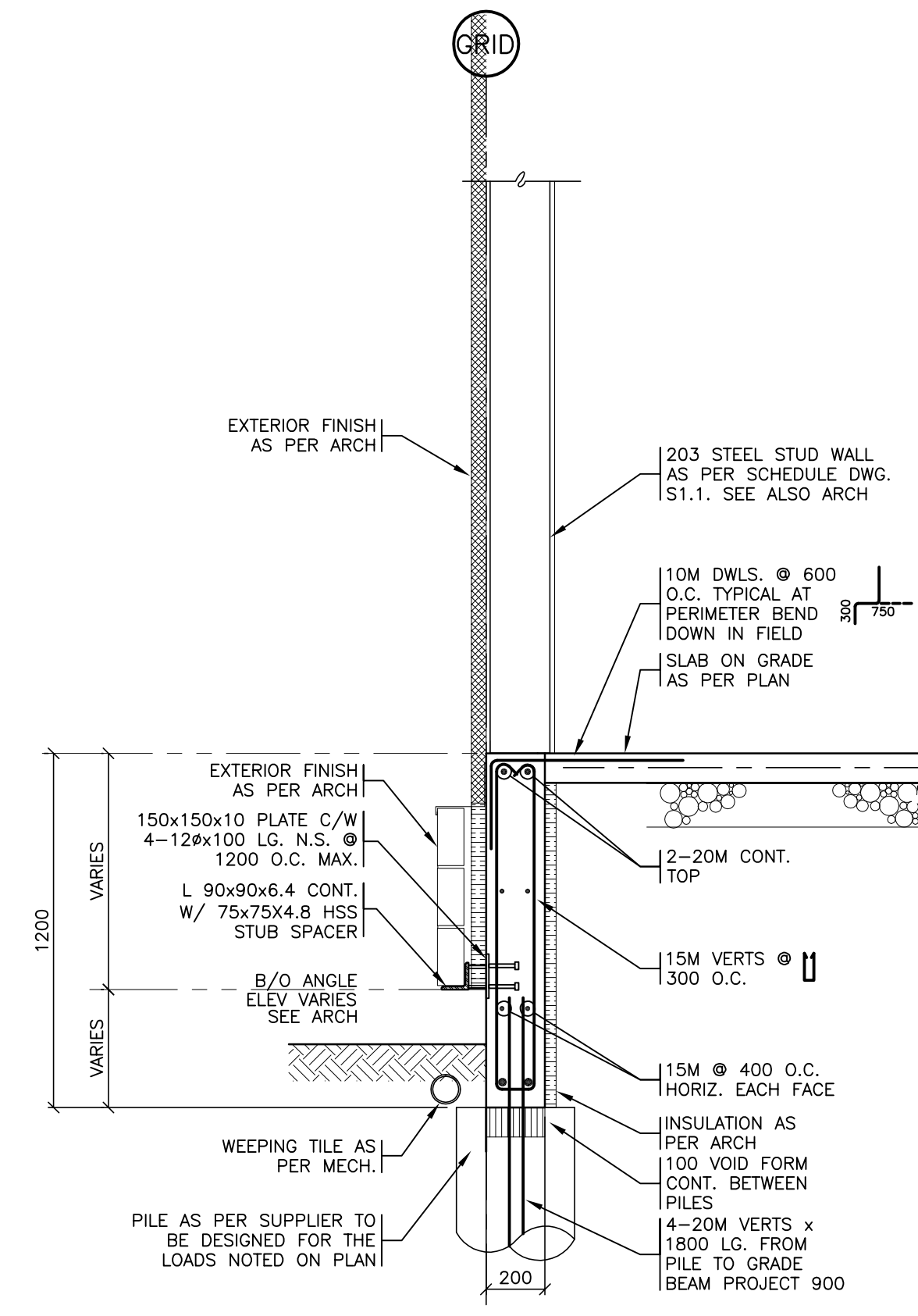
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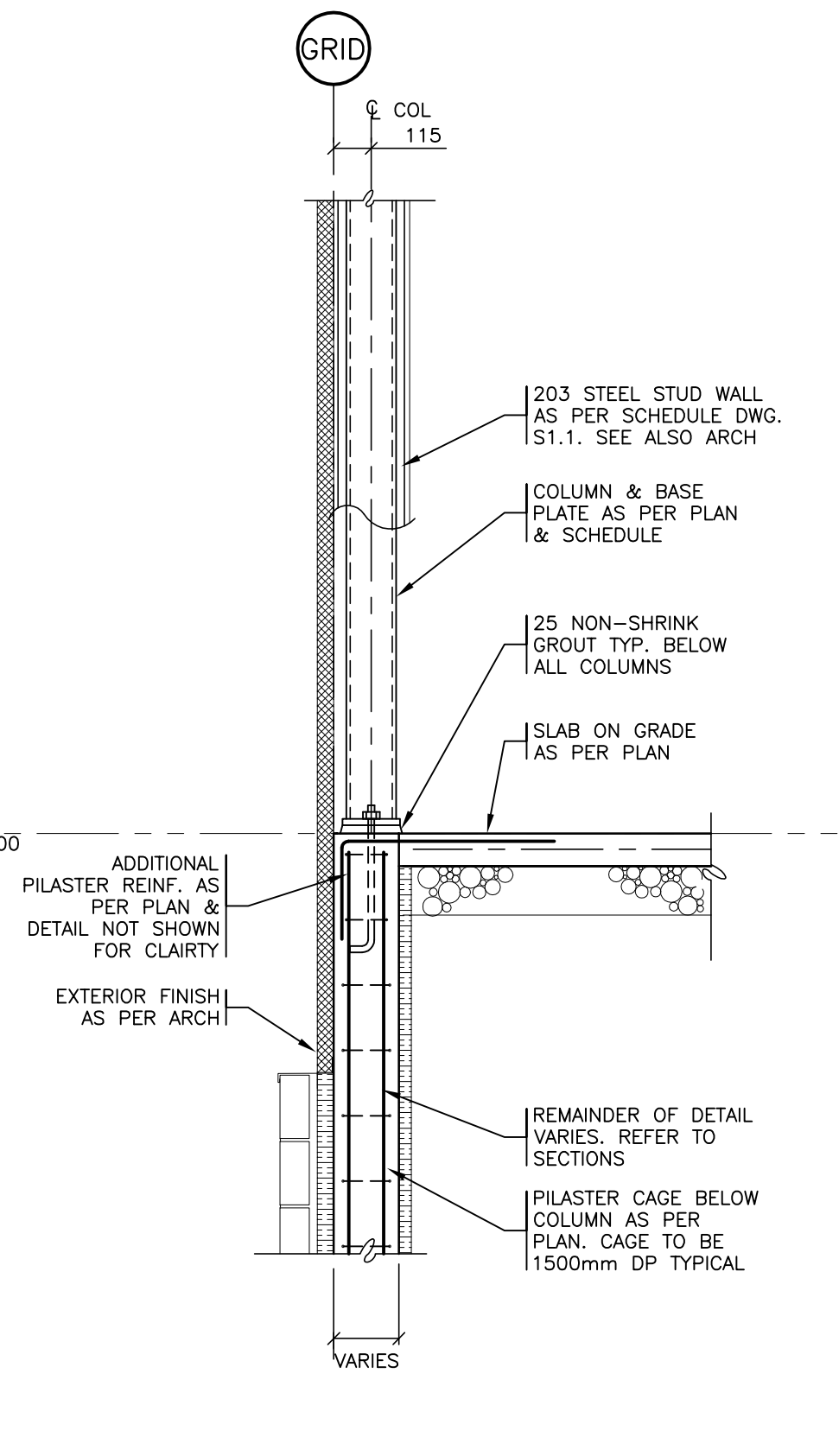
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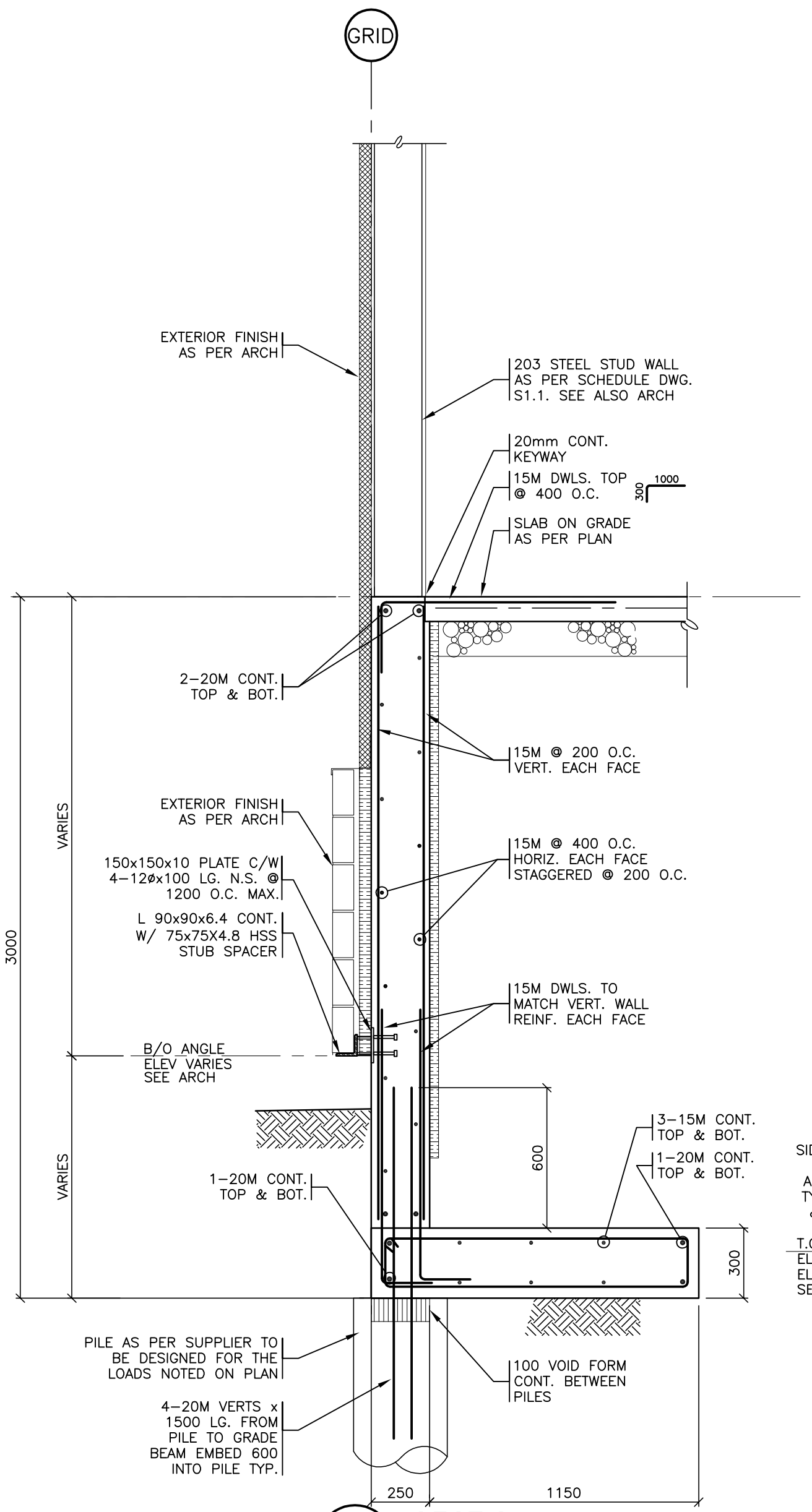
S4.1
2012-092



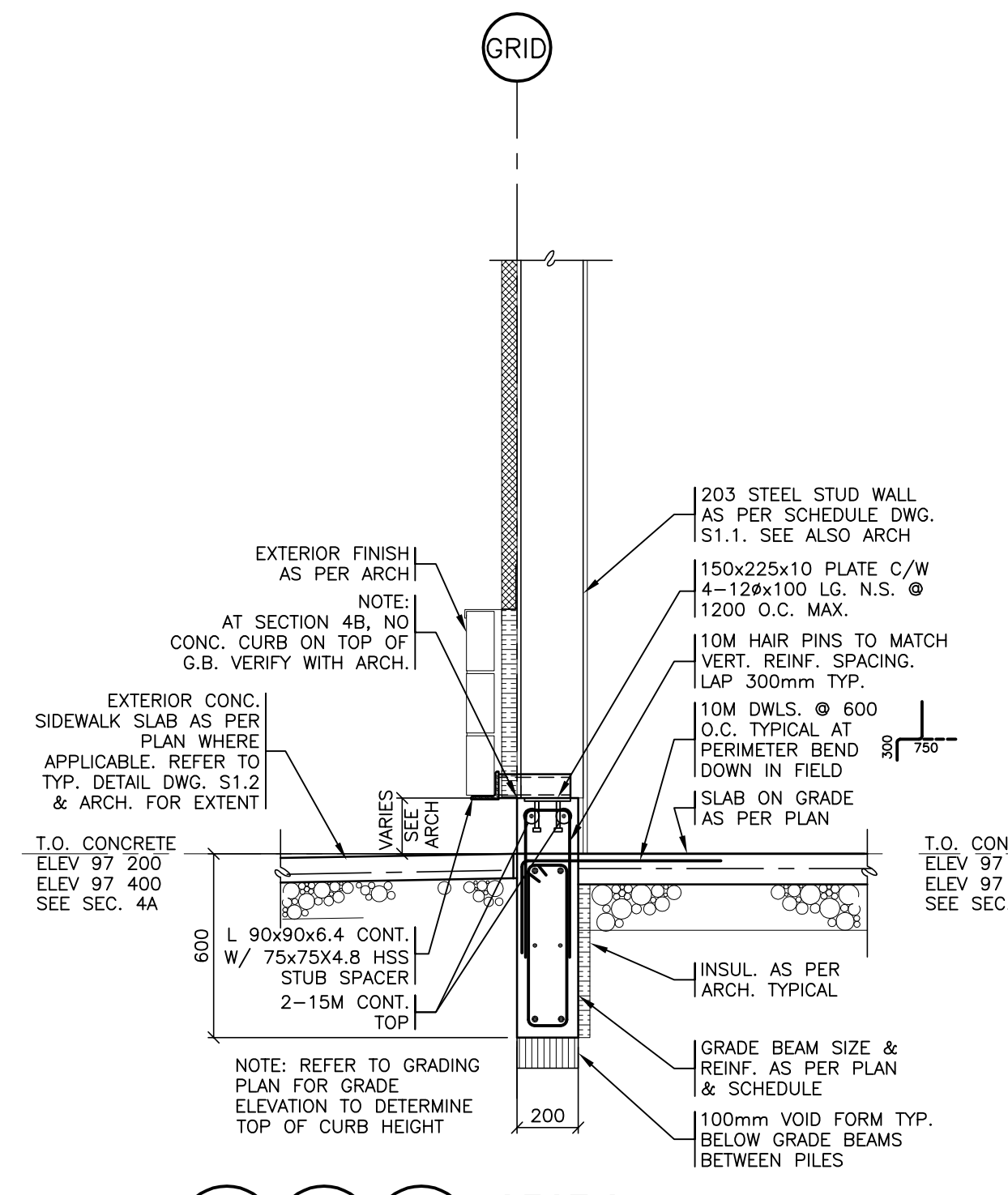
1 SECTION
S4.1 SCALE 1:20



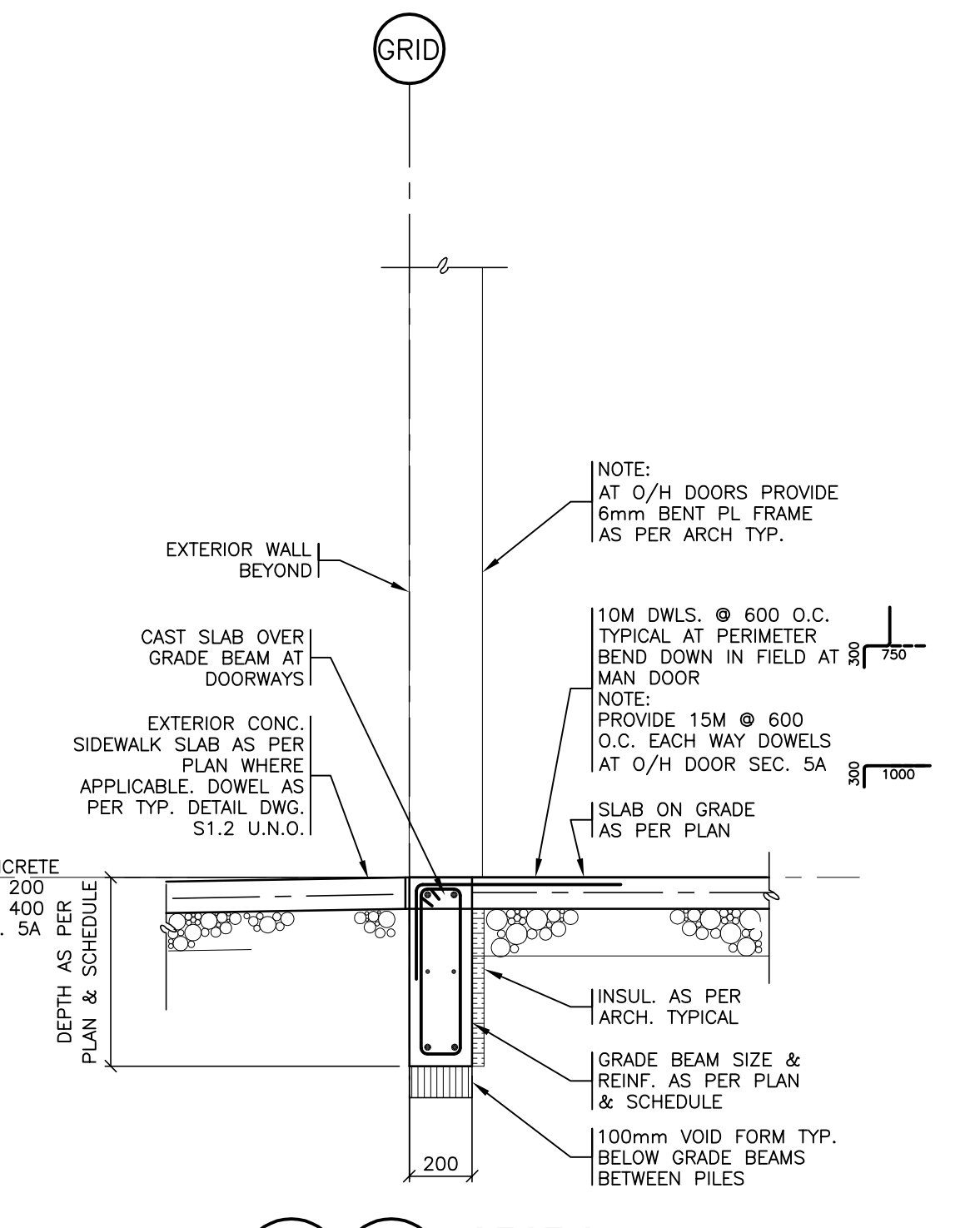
2 SECTION AT COLUMN
S4.1 SCALE 1:20



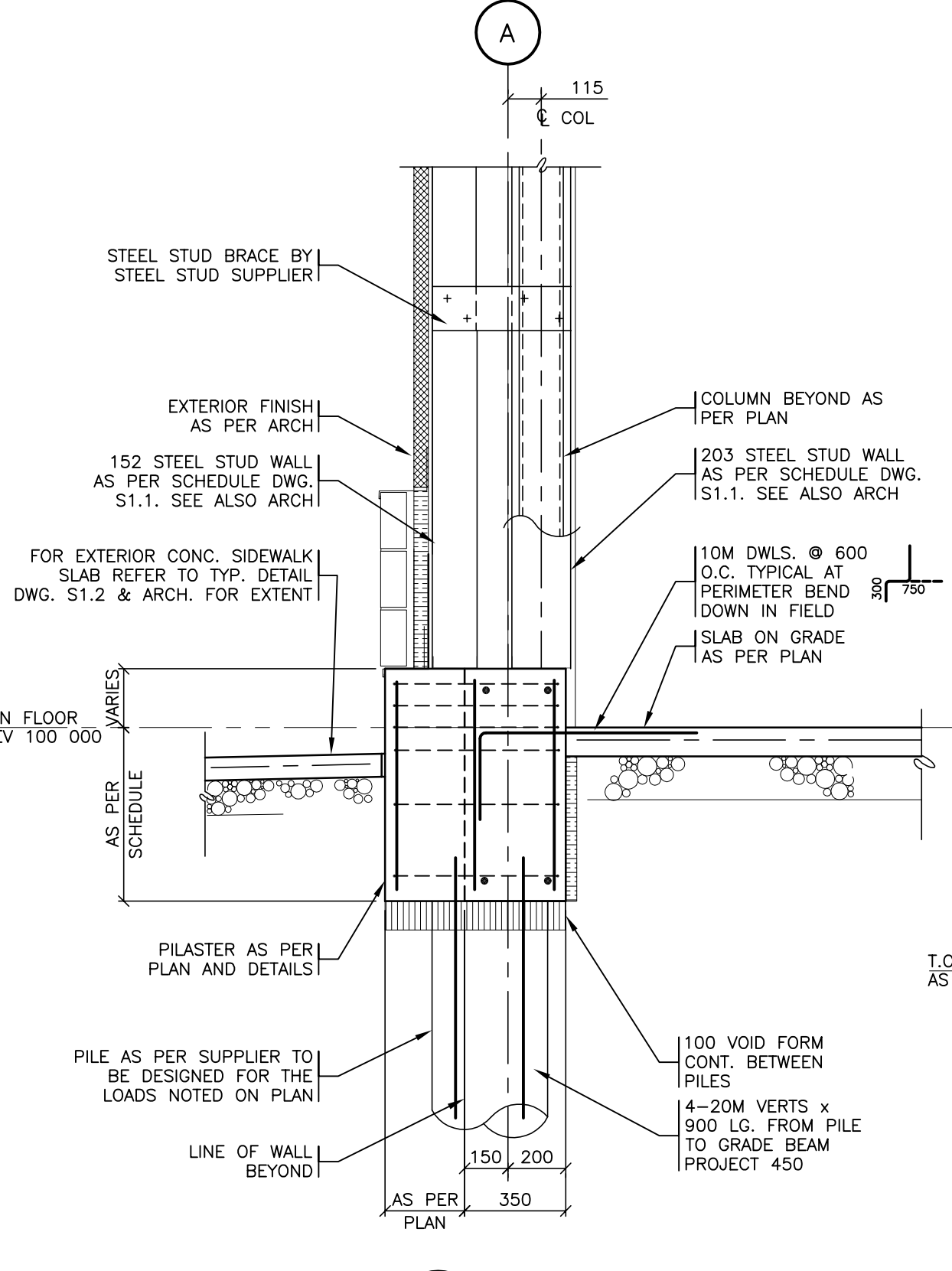
3 SECTION
S4.1 SCALE 1:20



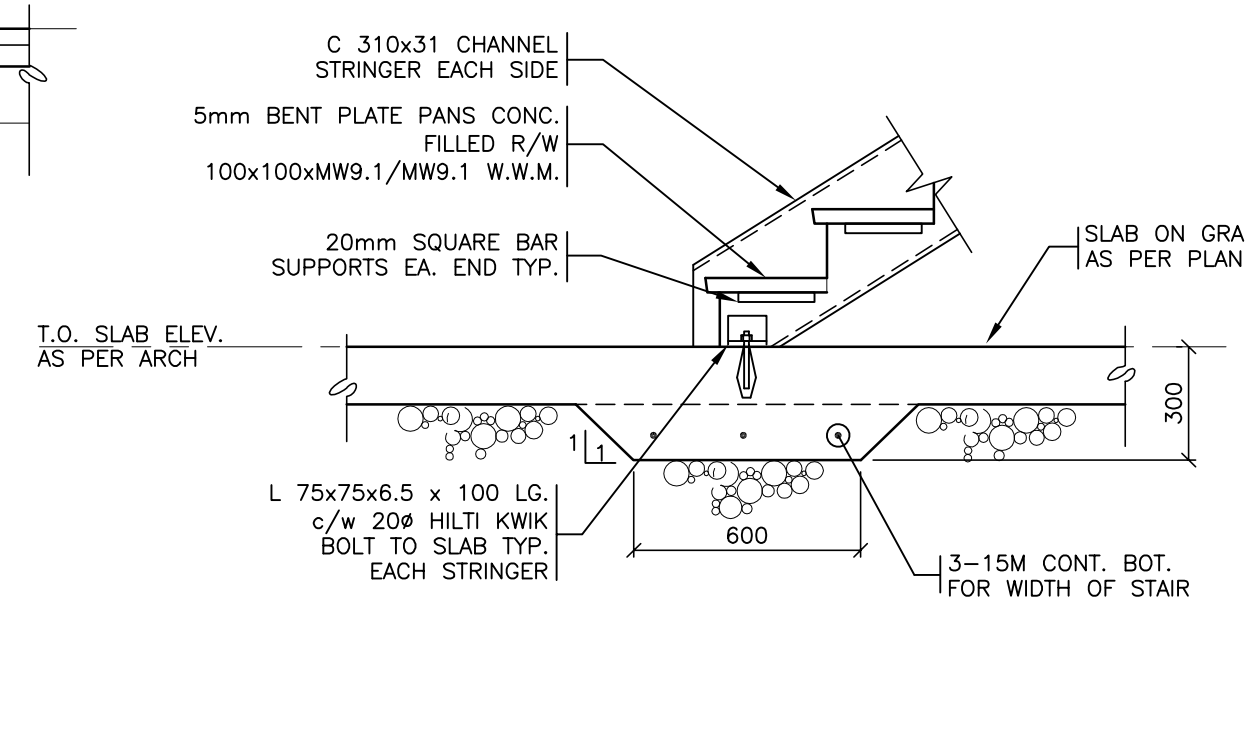
4A SECTION
S4.1 SCALE 1:20



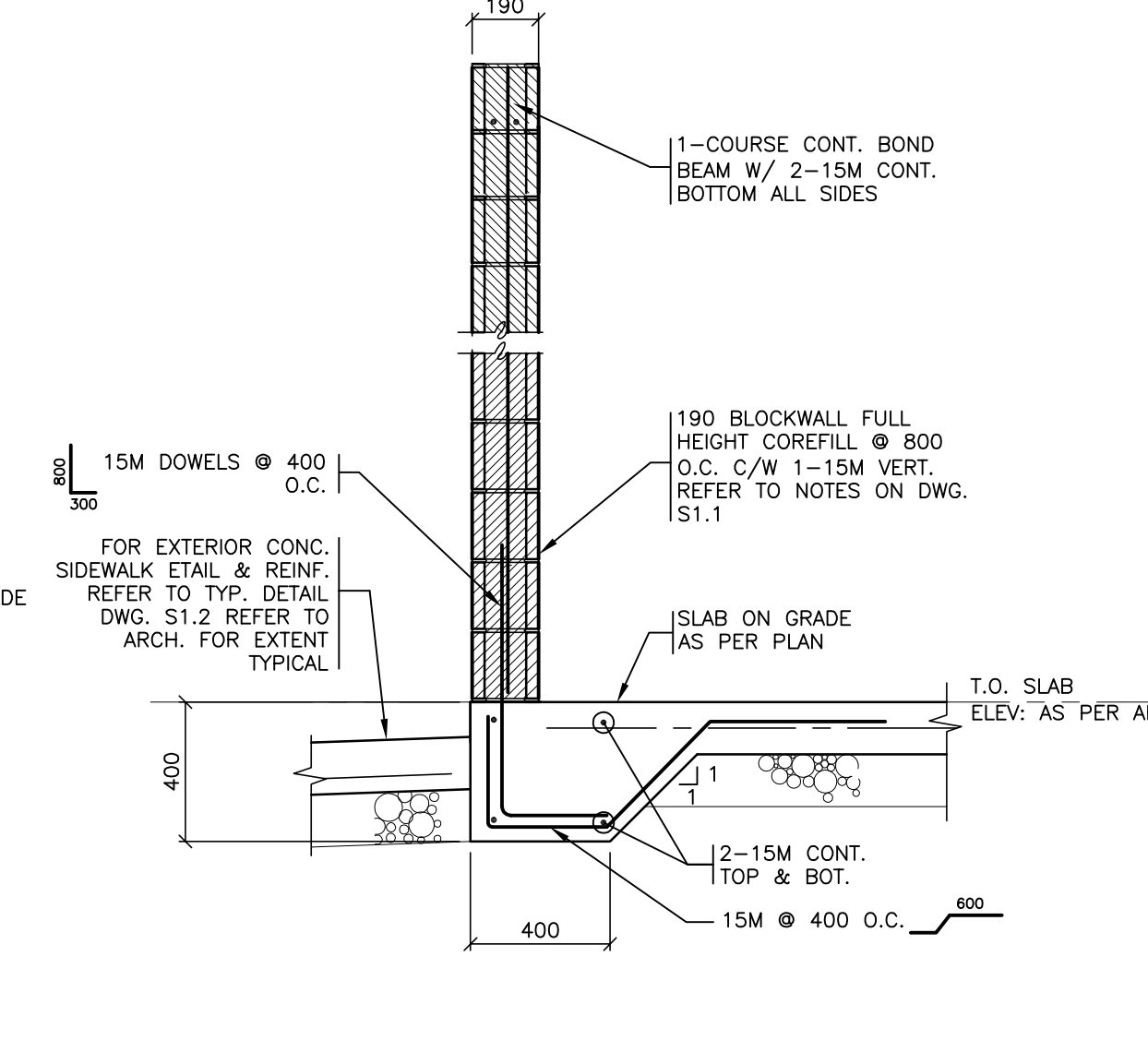
5 SECTION
S4.1 SCALE 1:20



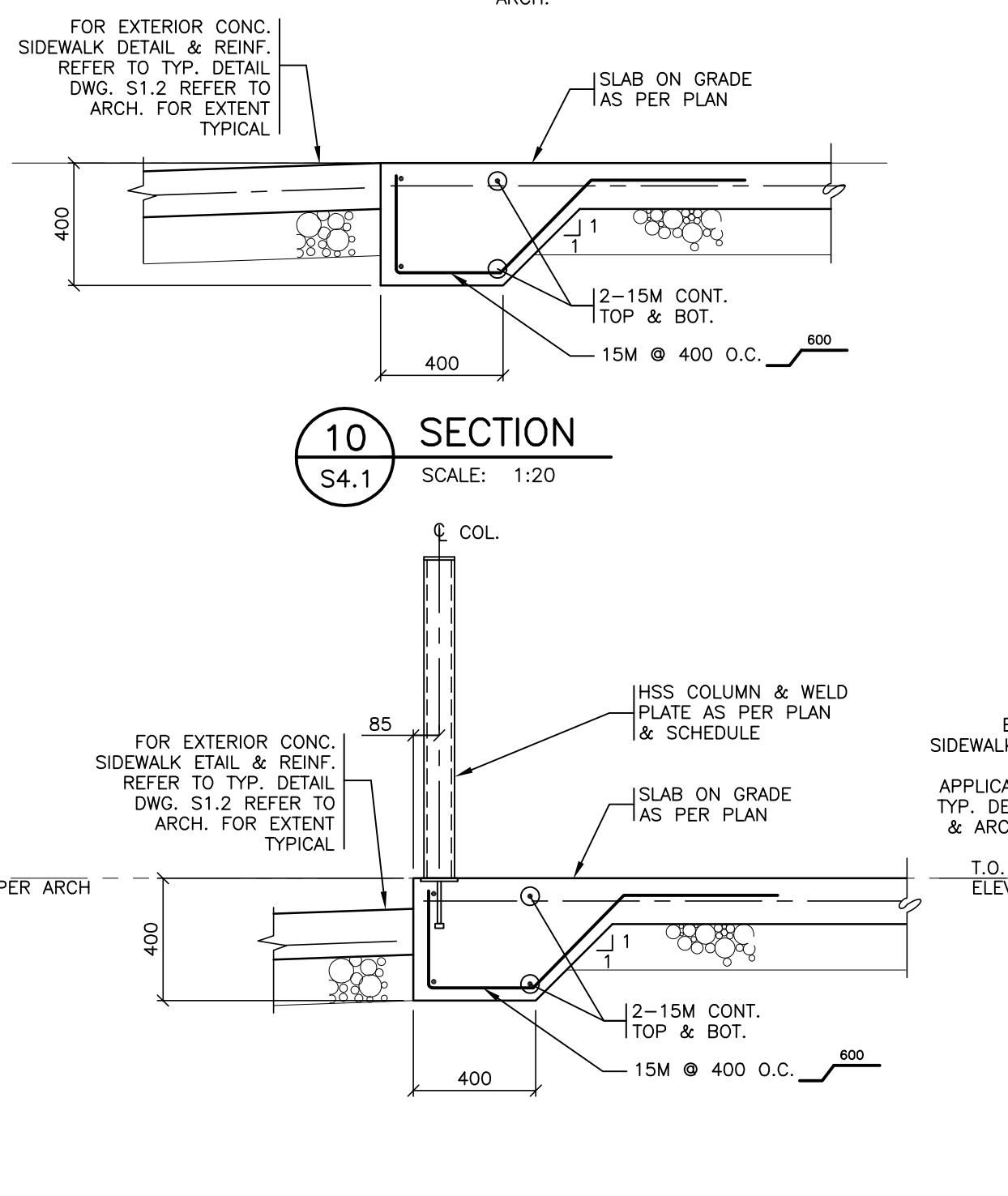
6 SECTION
S4.1 SCALE 1:20



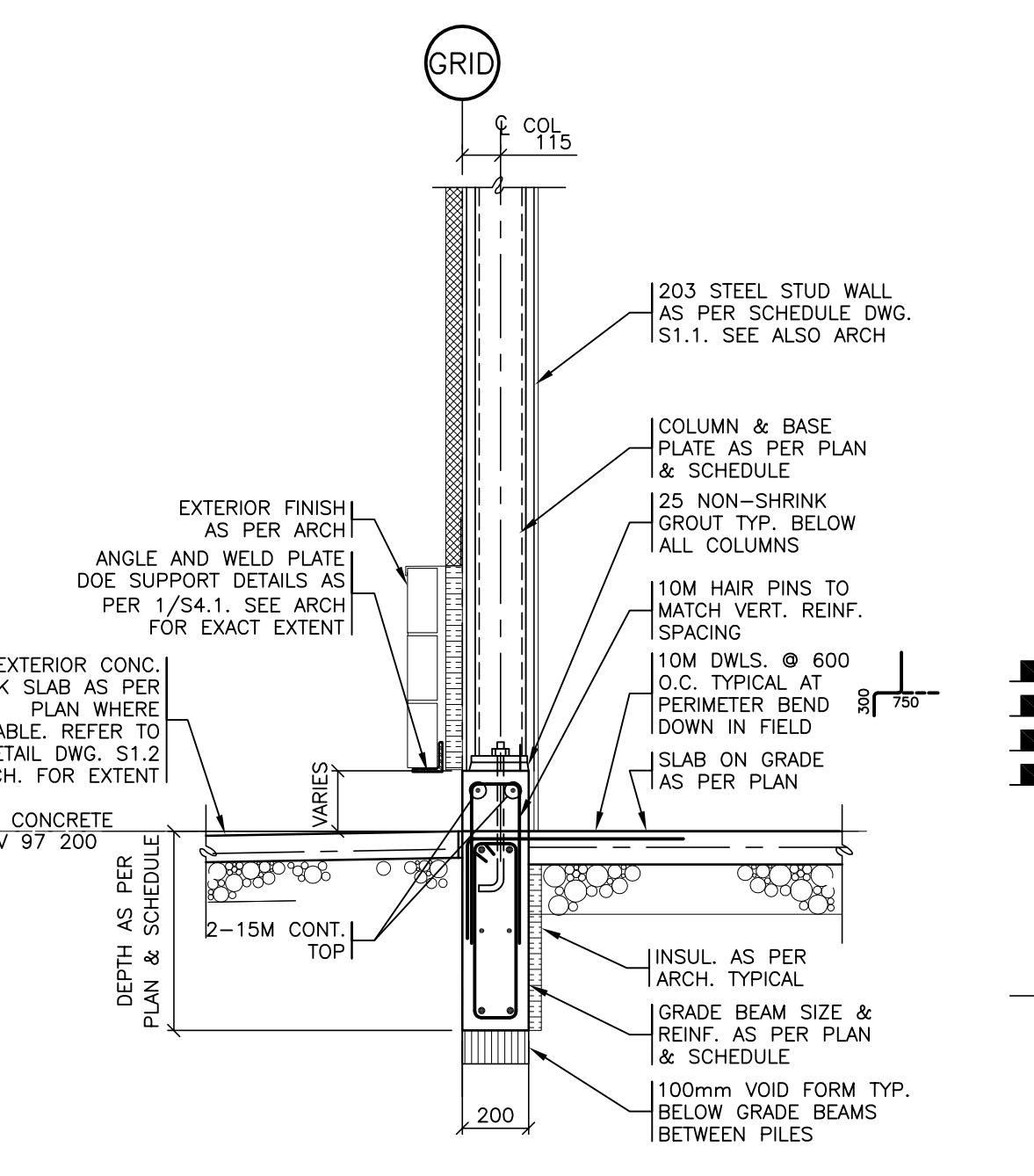
7 SECTION AT STAIR BASE
S4.1 SCALE 1:20



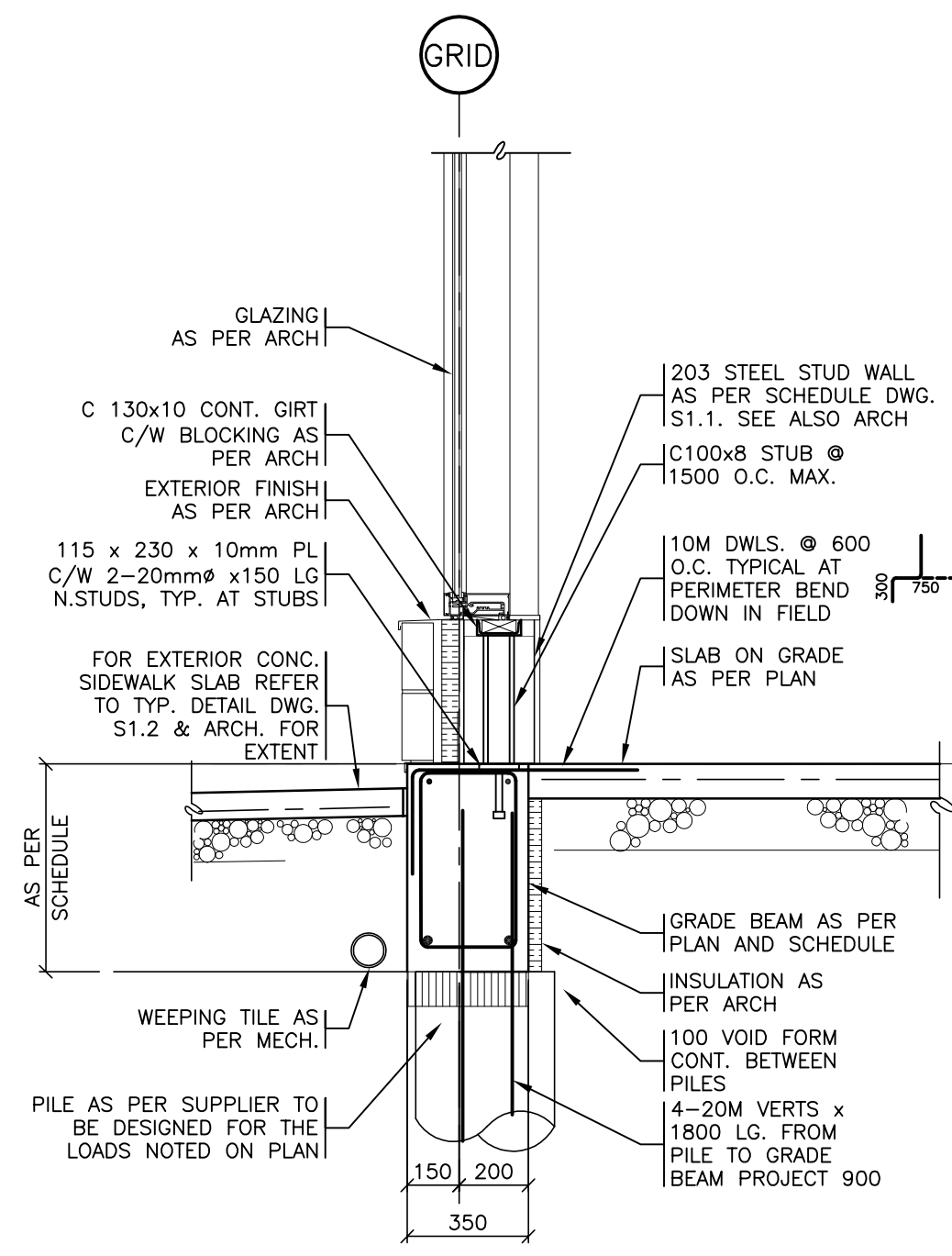
8 SECTION AT GARBAGE ENCL.
S4.1 SCALE 1:20



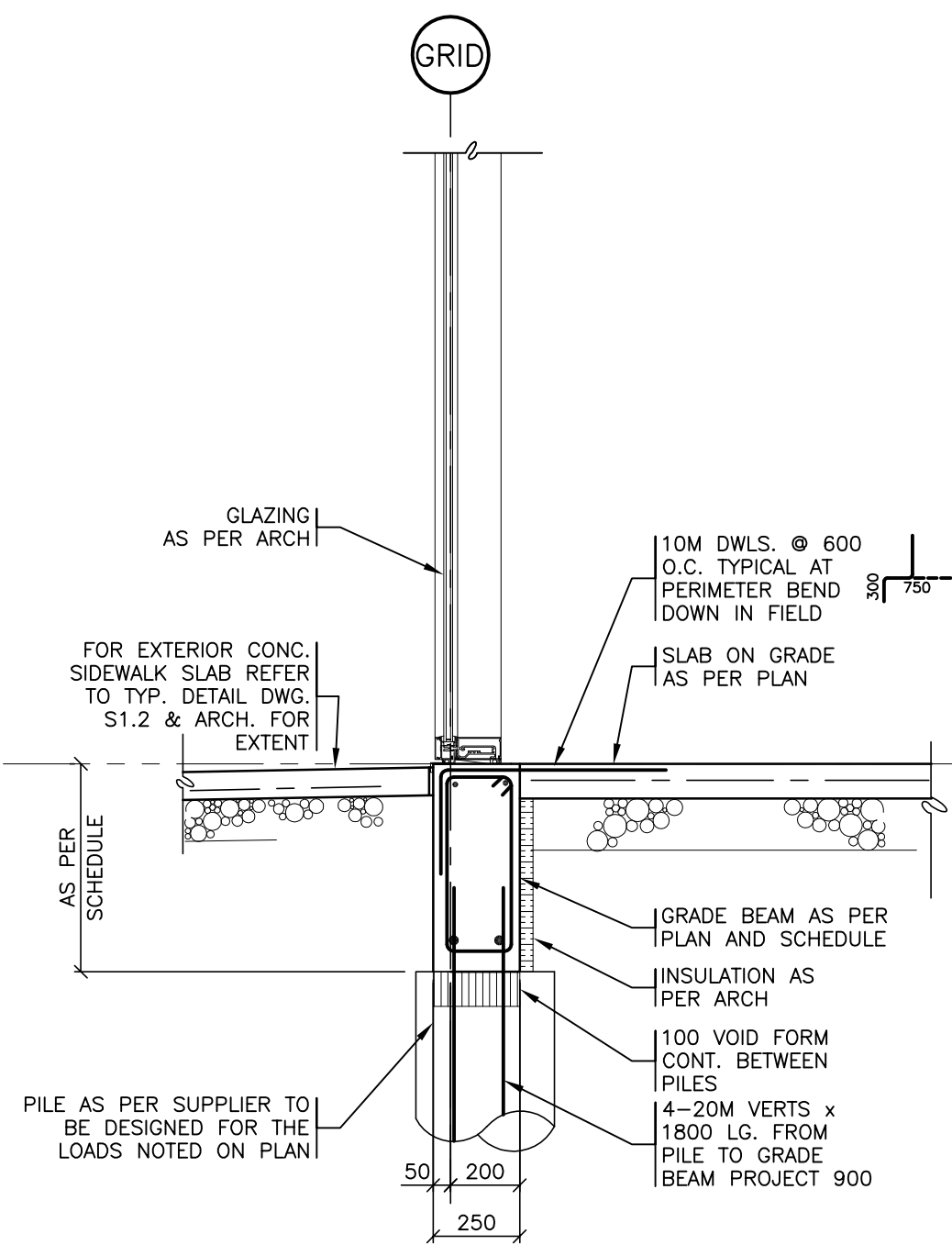
9 SECTION AT GARBAGE ENCL. COLUMN
S4.1 SCALE 1:20



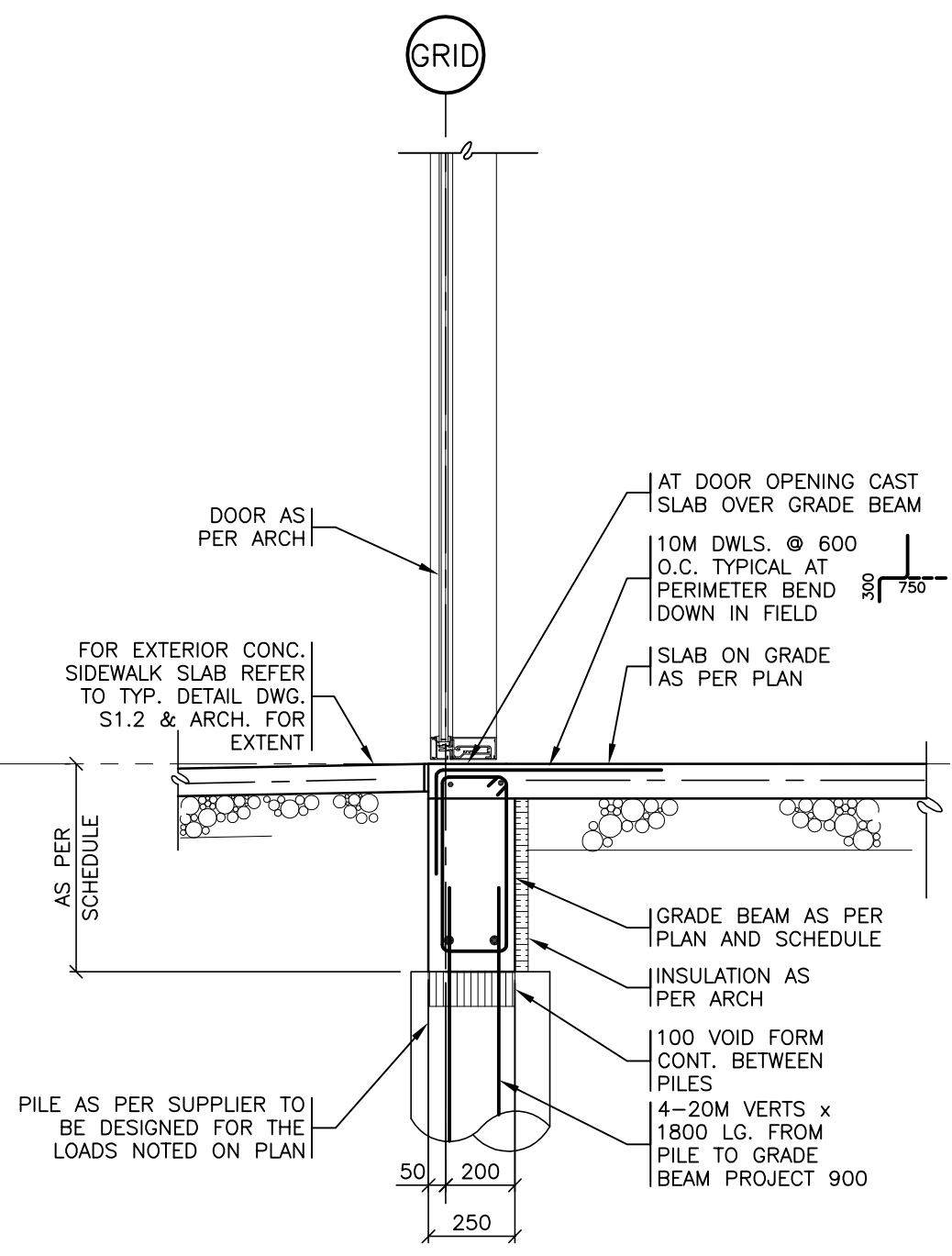
10 SECTION AT COL.
S4.1 SCALE 1:20



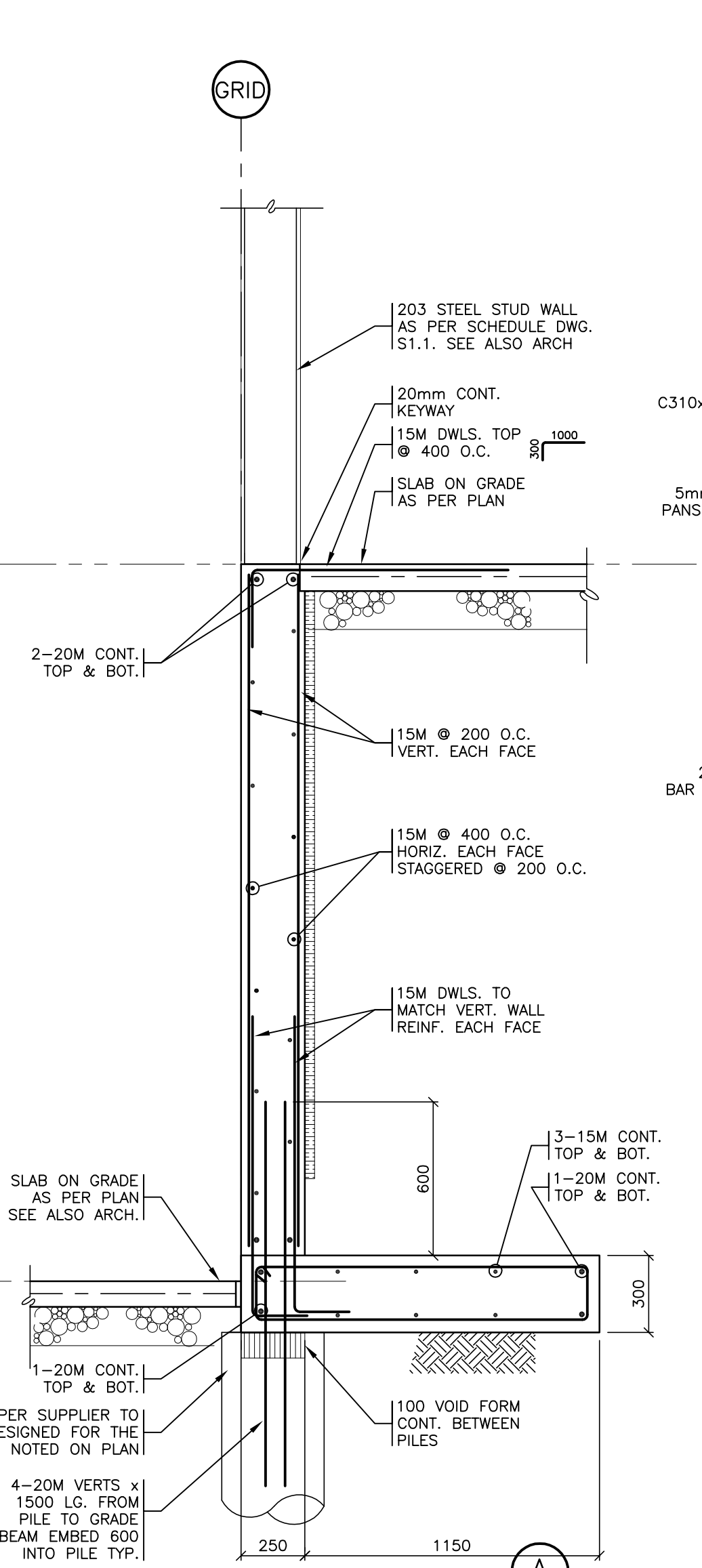
1 SECTION AT GLAZING
S4.3 SCALE 1:20



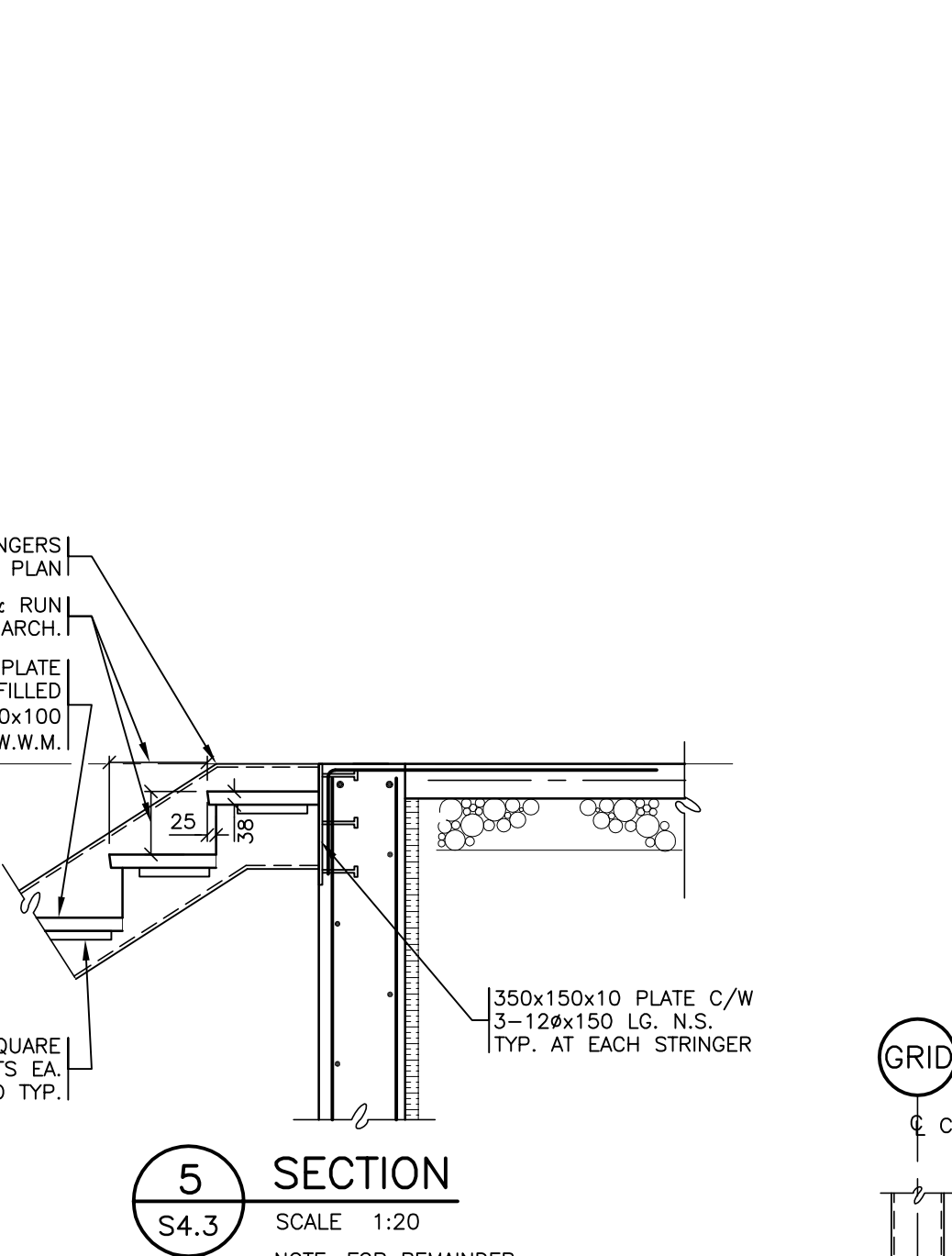
2 SECTION AT GLAZING
S4.3 SCALE 1:20



3 SECTION AT DOOR
S4.3 SCALE 1:20



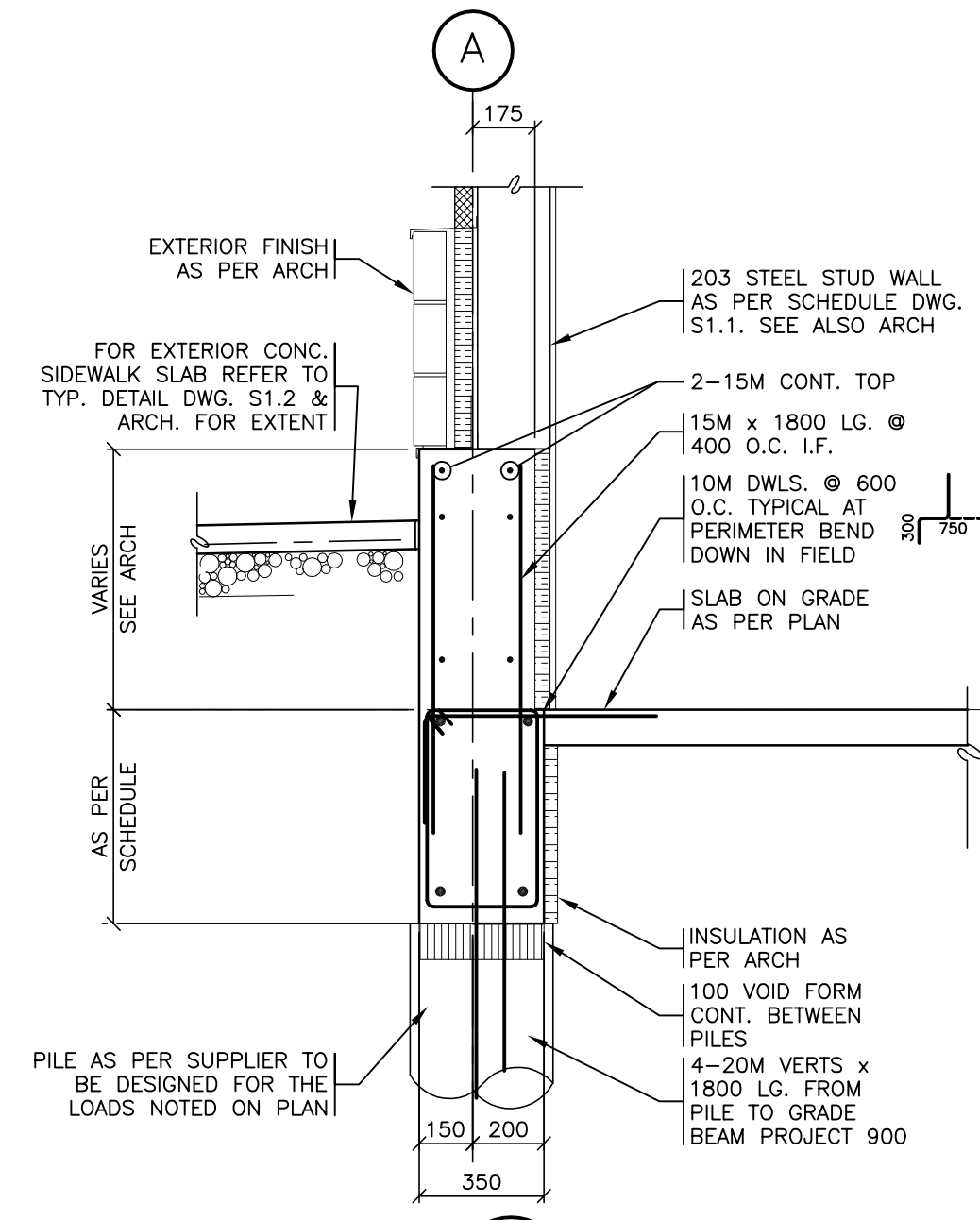
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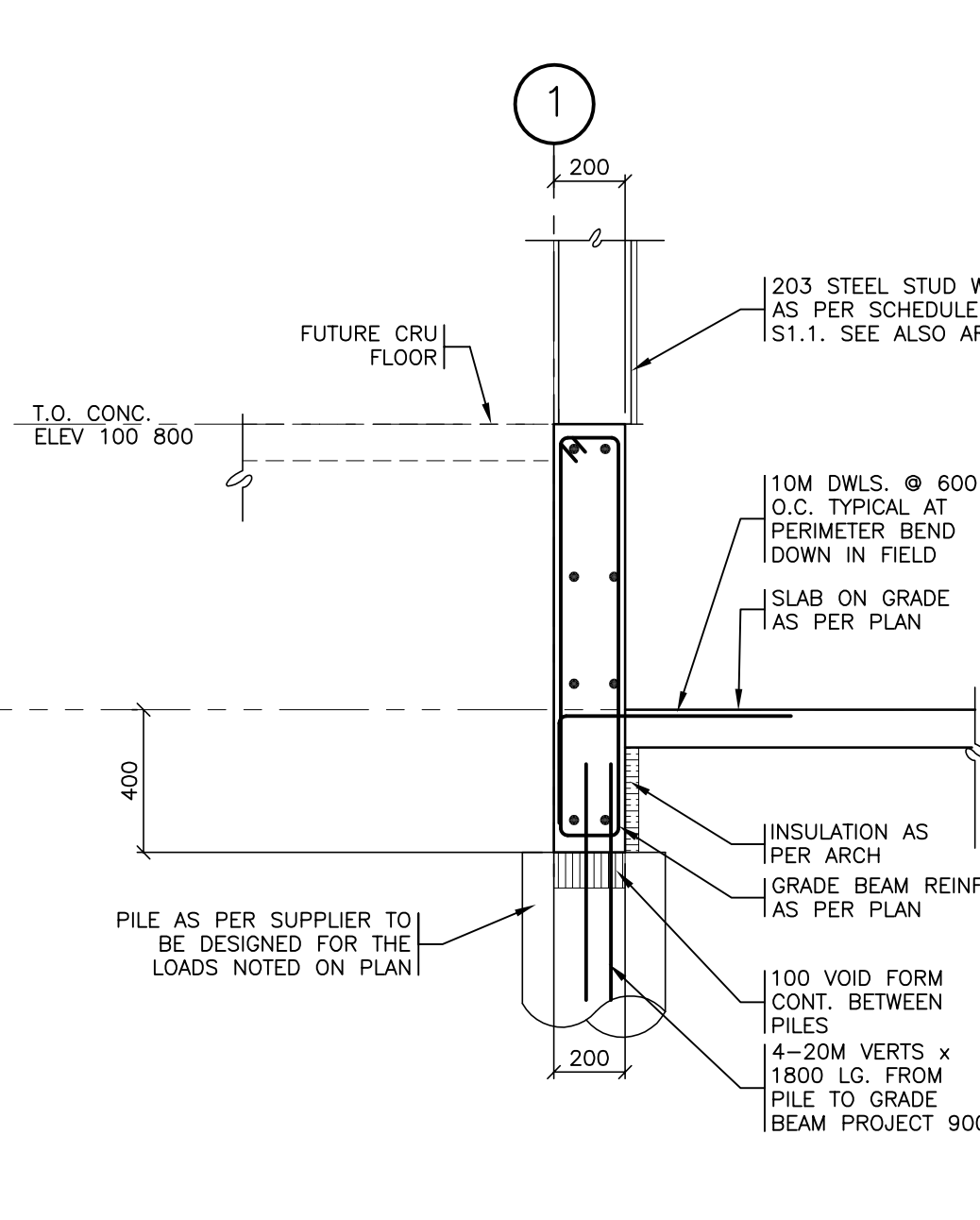
5 SECTION
S4.3 SCALE 1:20



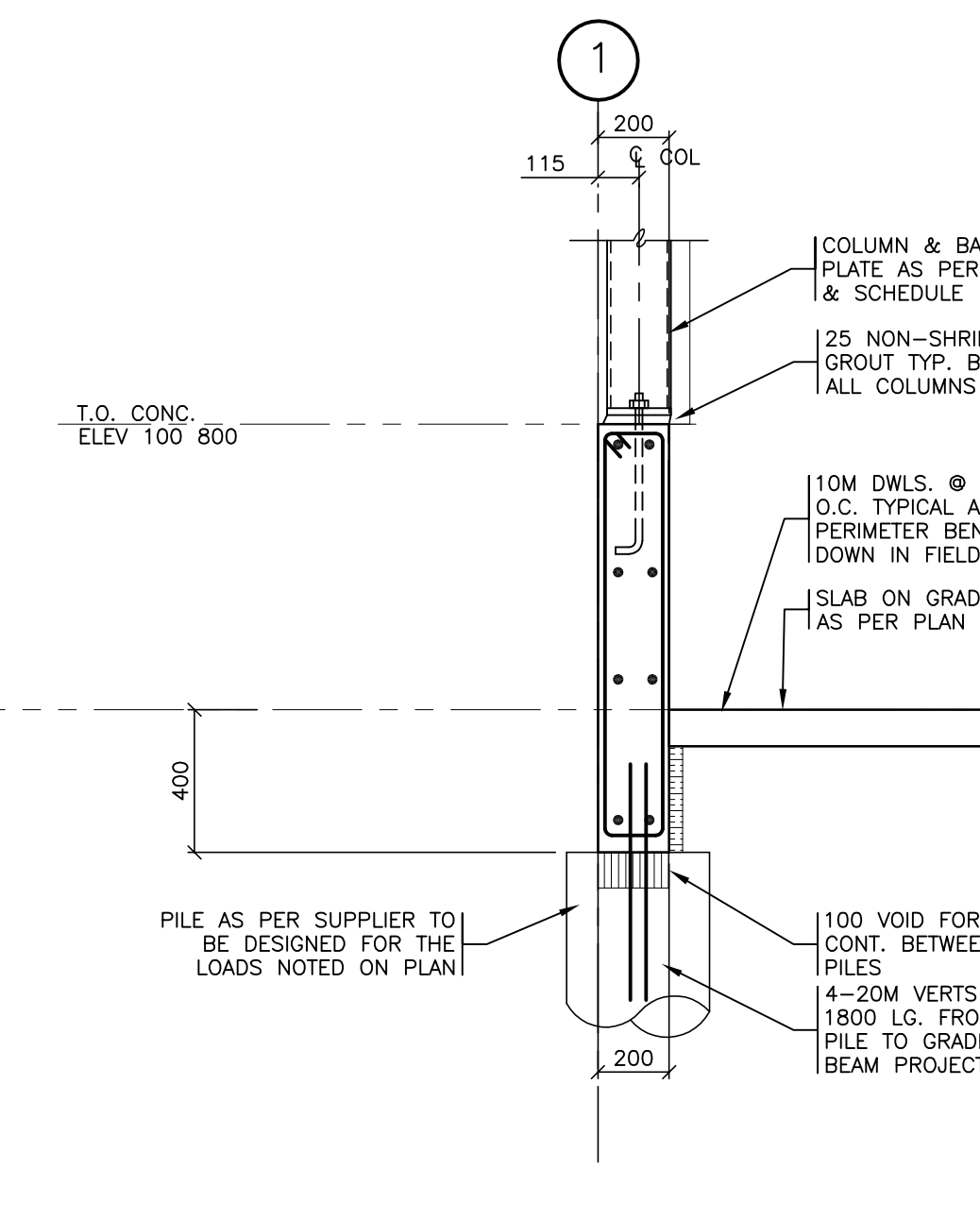
6A SECTION
S4.3 SCALE 1:20



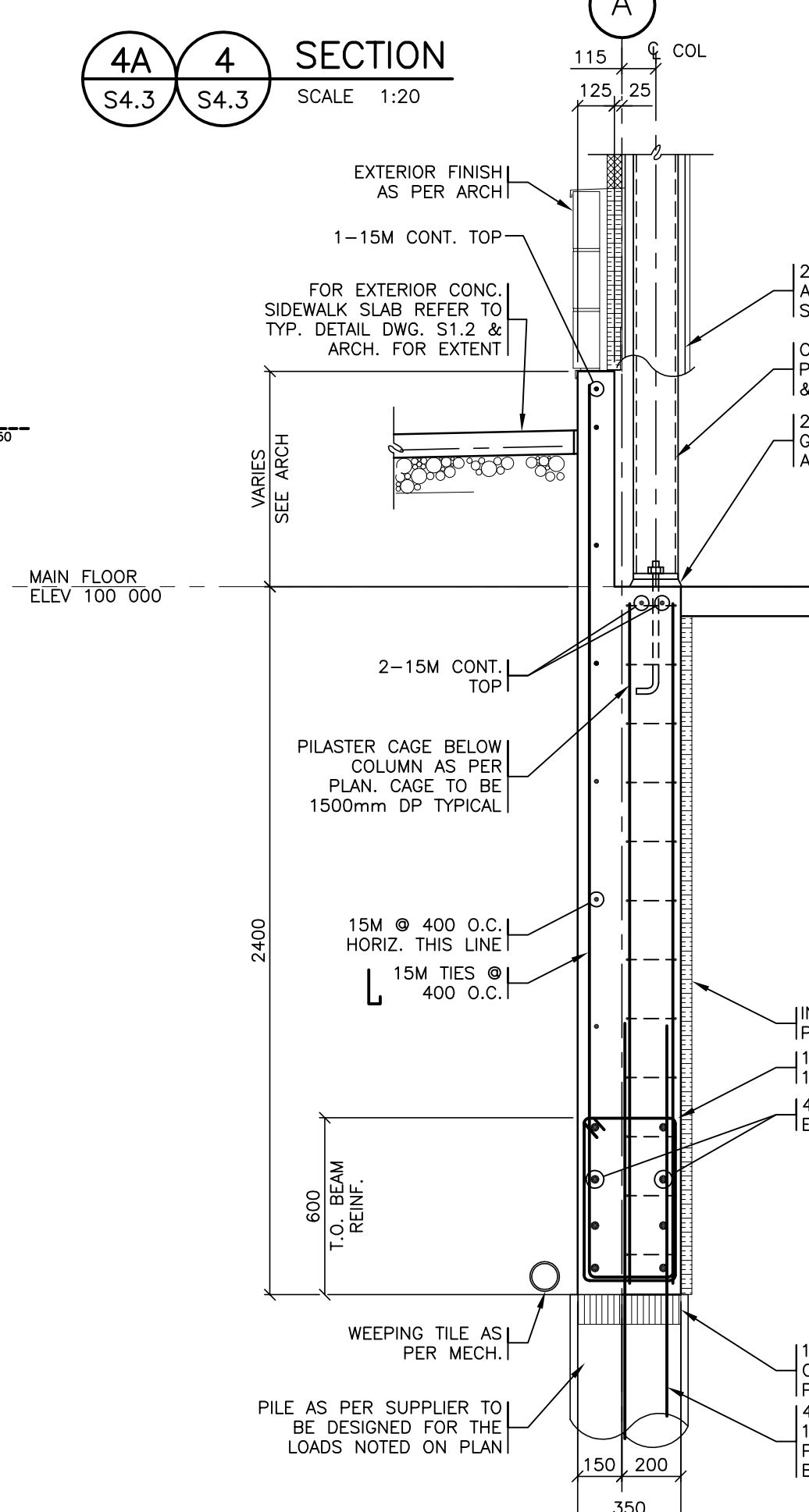
7 SECTION
S4.3 SCALE 1:20



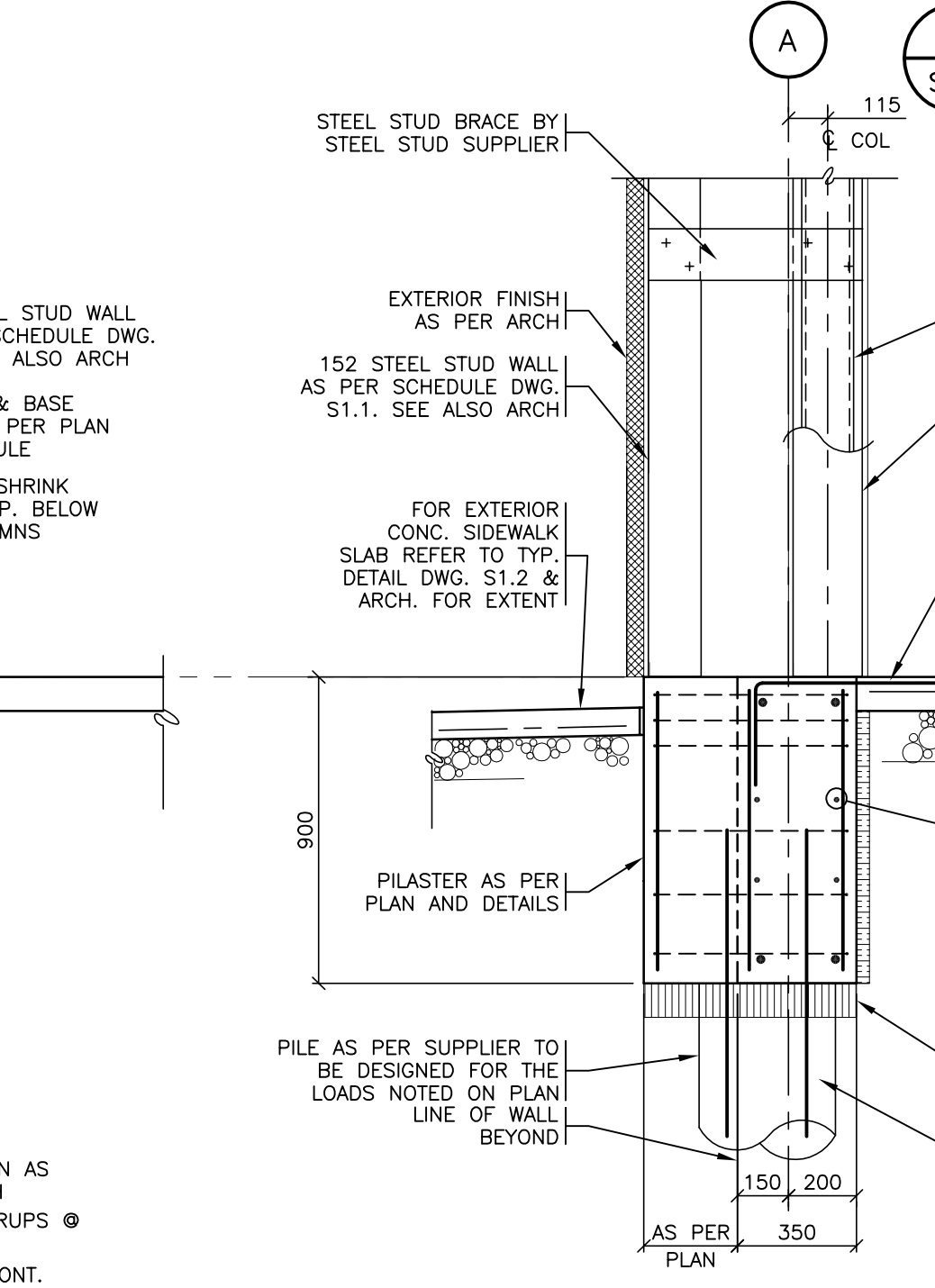
8 SECTION
S4.3 SCALE 1:20



9 SECTION
S4.3 SCALE 1:20



10 SECTION
S4.3 SCALE 1:20



11 SECTION
S4.3 SCALE 1:20



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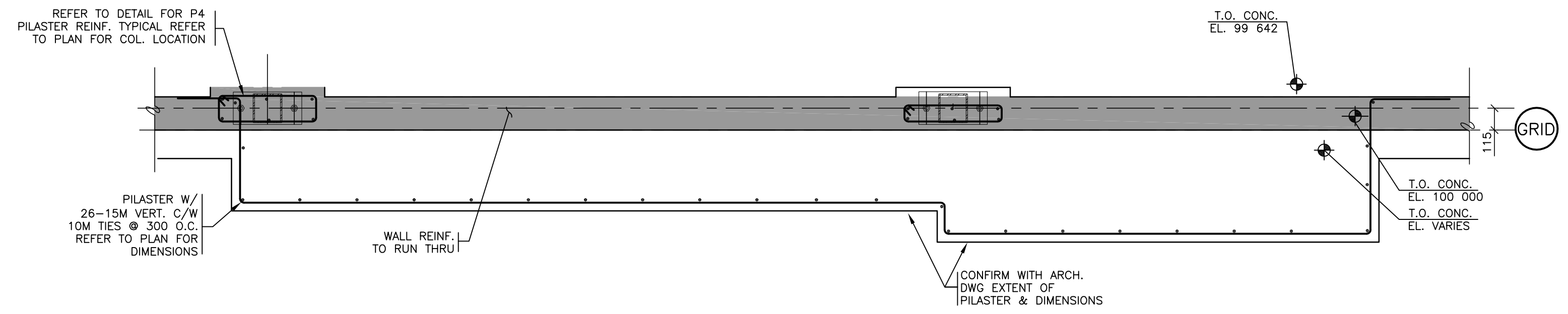
VISIONS FOUNDATION SECTIONS

S4.3
2012-092

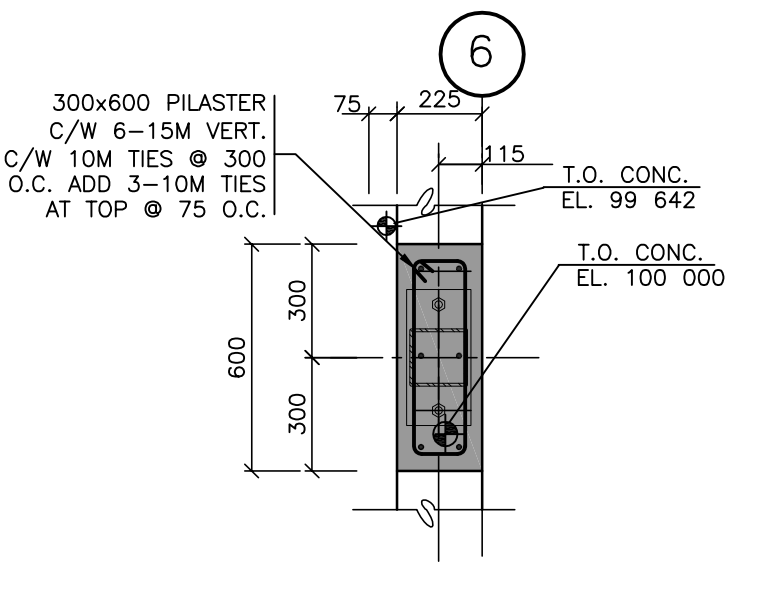


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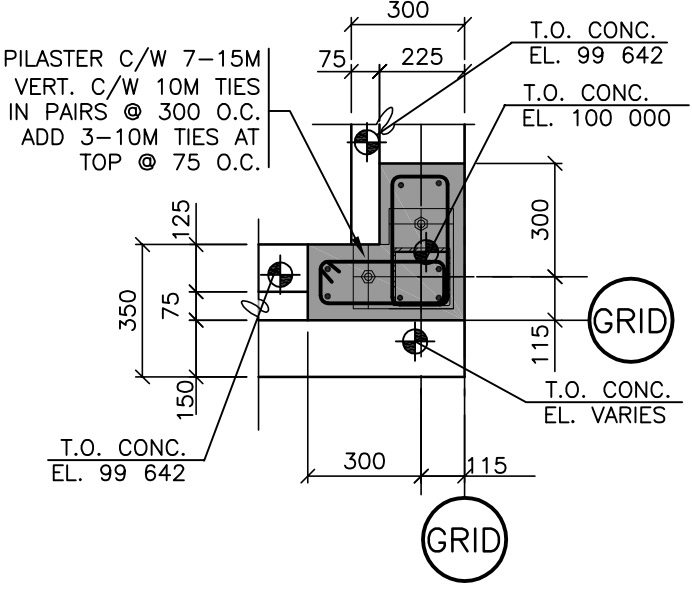
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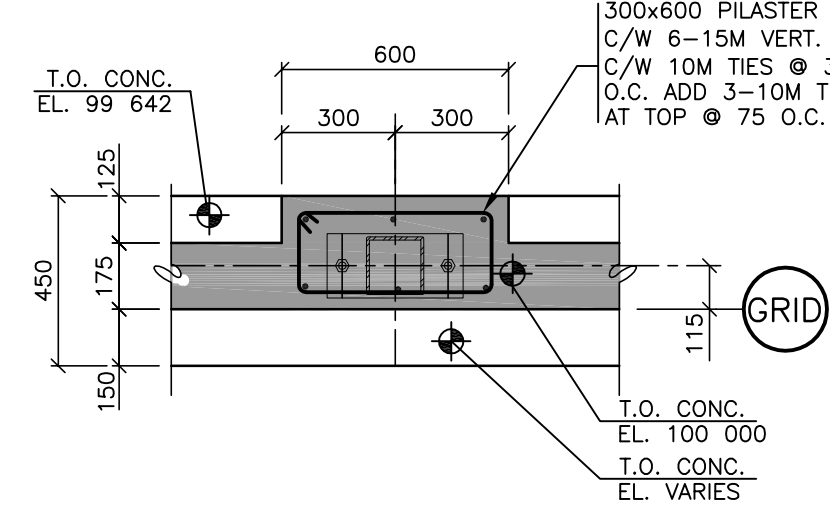
P1 PILASTER DETAIL
 S4.4 SCALE 1:20
 NOTE:
 ORIENTATION VARIES
 REFER TO PLAN.



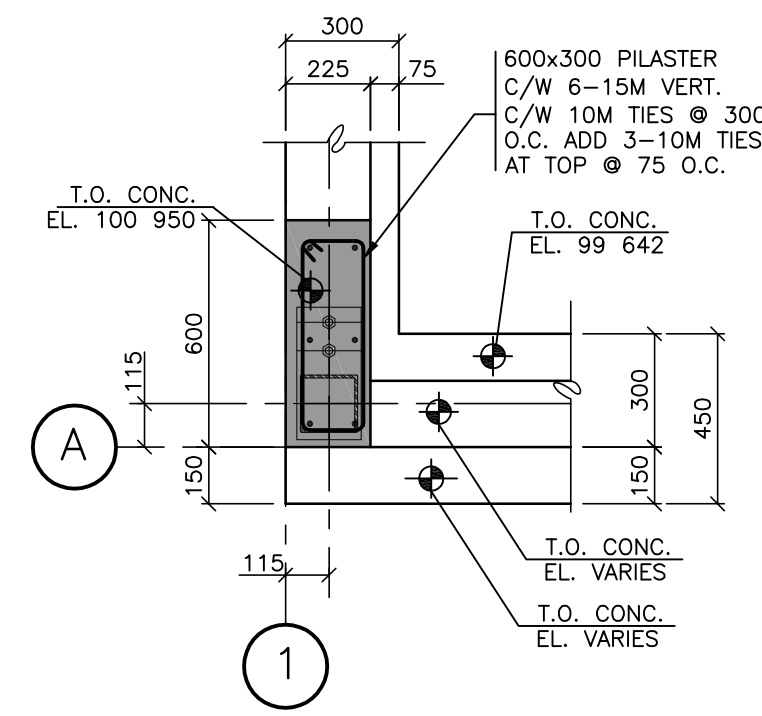
P2 PILASTER DETAIL
 S4.4 SCALE 1:20



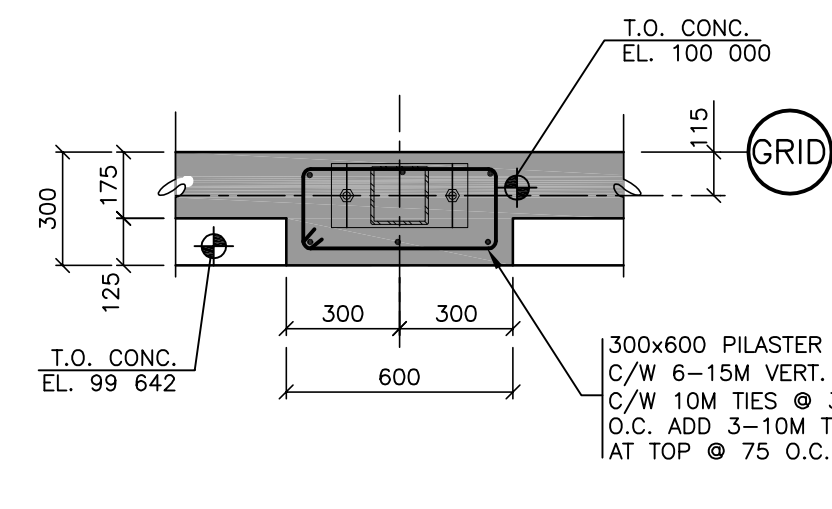
P3 PILASTER DETAIL
 S4.4 SCALE 1:20
 NOTE:
 ORIENTATION VARIES
 REFER TO PLAN.



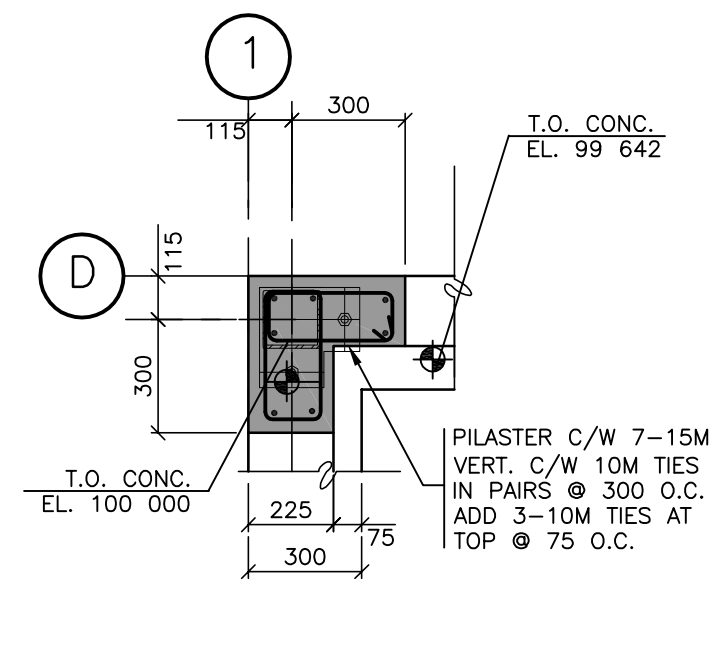
P4 PILASTER DETAIL
 S4.4 SCALE 1:20



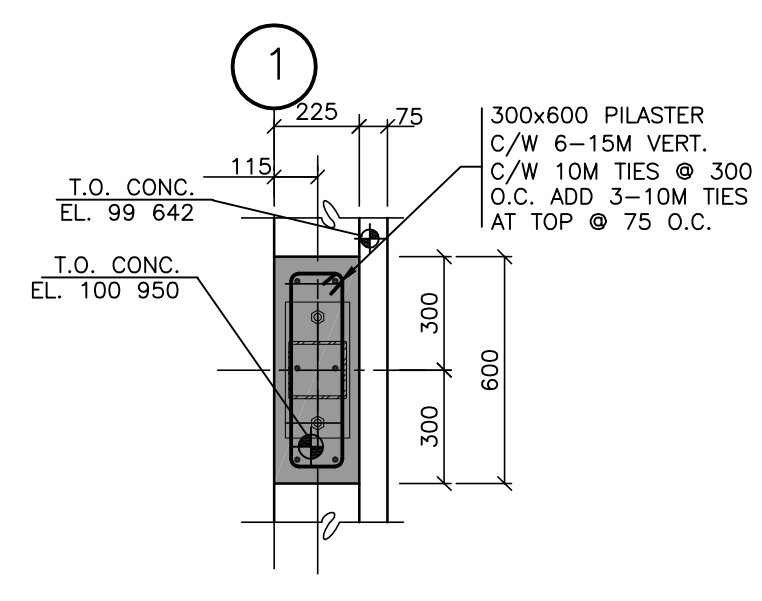
P5 PILASTER DETAIL
 S4.4 SCALE 1:20



P6 PILASTER DETAIL
 S4.4 SCALE 1:20



P7 PILASTER DETAIL
 S4.4 SCALE 1:20



P8 PILASTER DETAIL
 S4.4 SCALE 1:20

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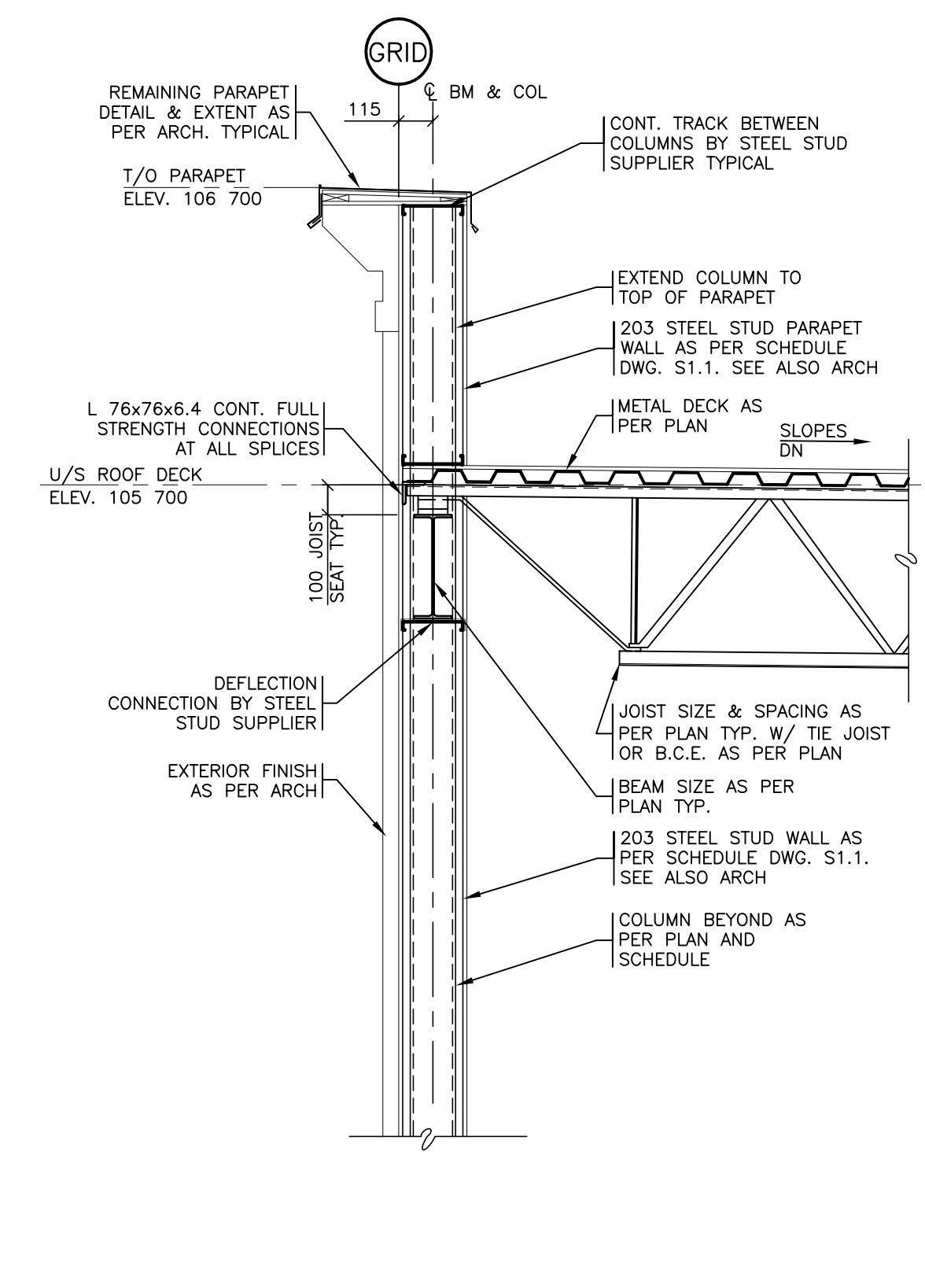
VISIONS
 PILASTER DETAILS

S4.4
 2012-092

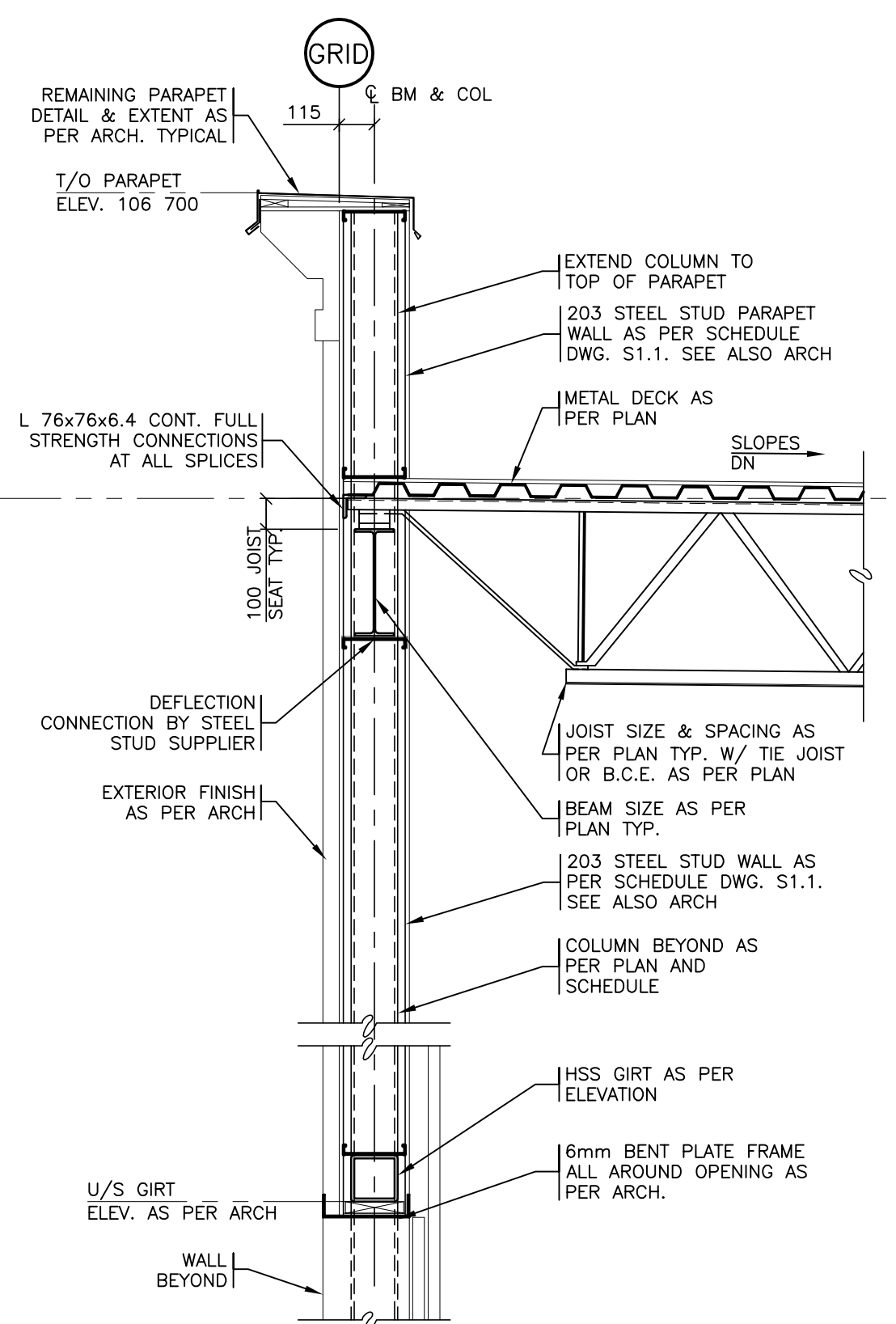


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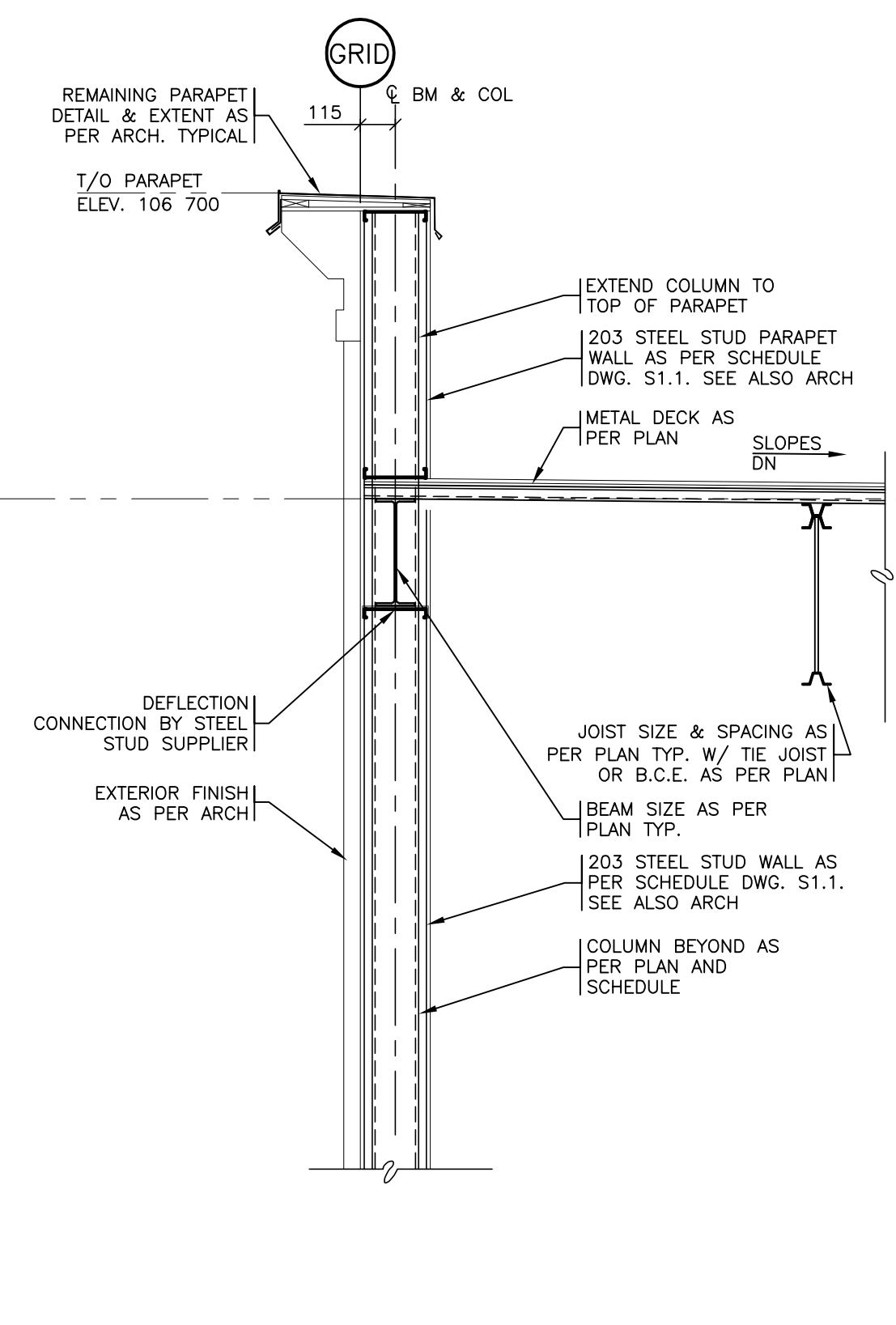
AK & ASSOCIATES
 lomsook-roney-tille & associates ltd.
 consulting structural engineers
 100 - 145 10th Avenue S.W. - Calgary - Alberta - T2C 0T7
 Phone: (403)244-4944 - Fax: (403)299-3110



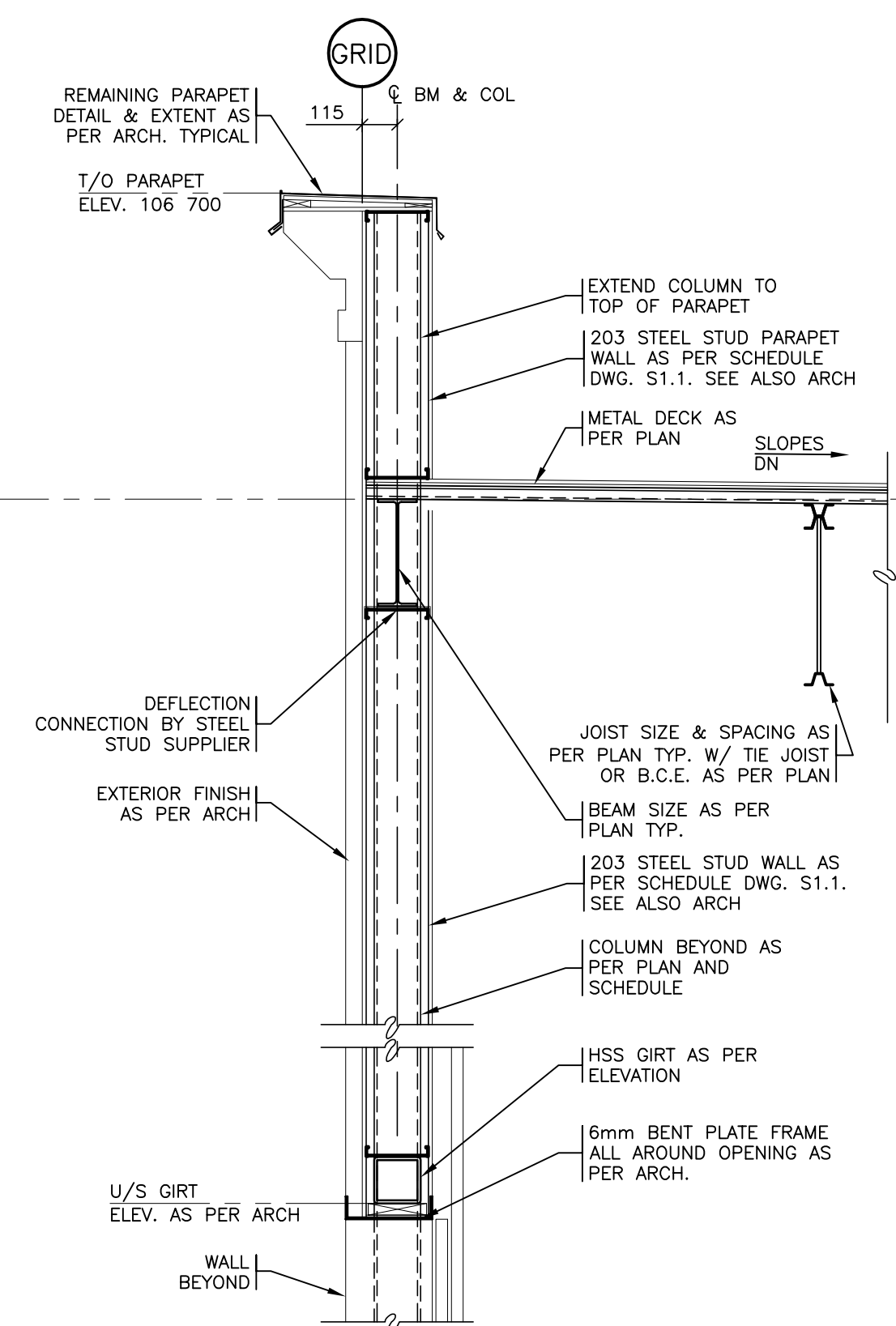
1 SECTION
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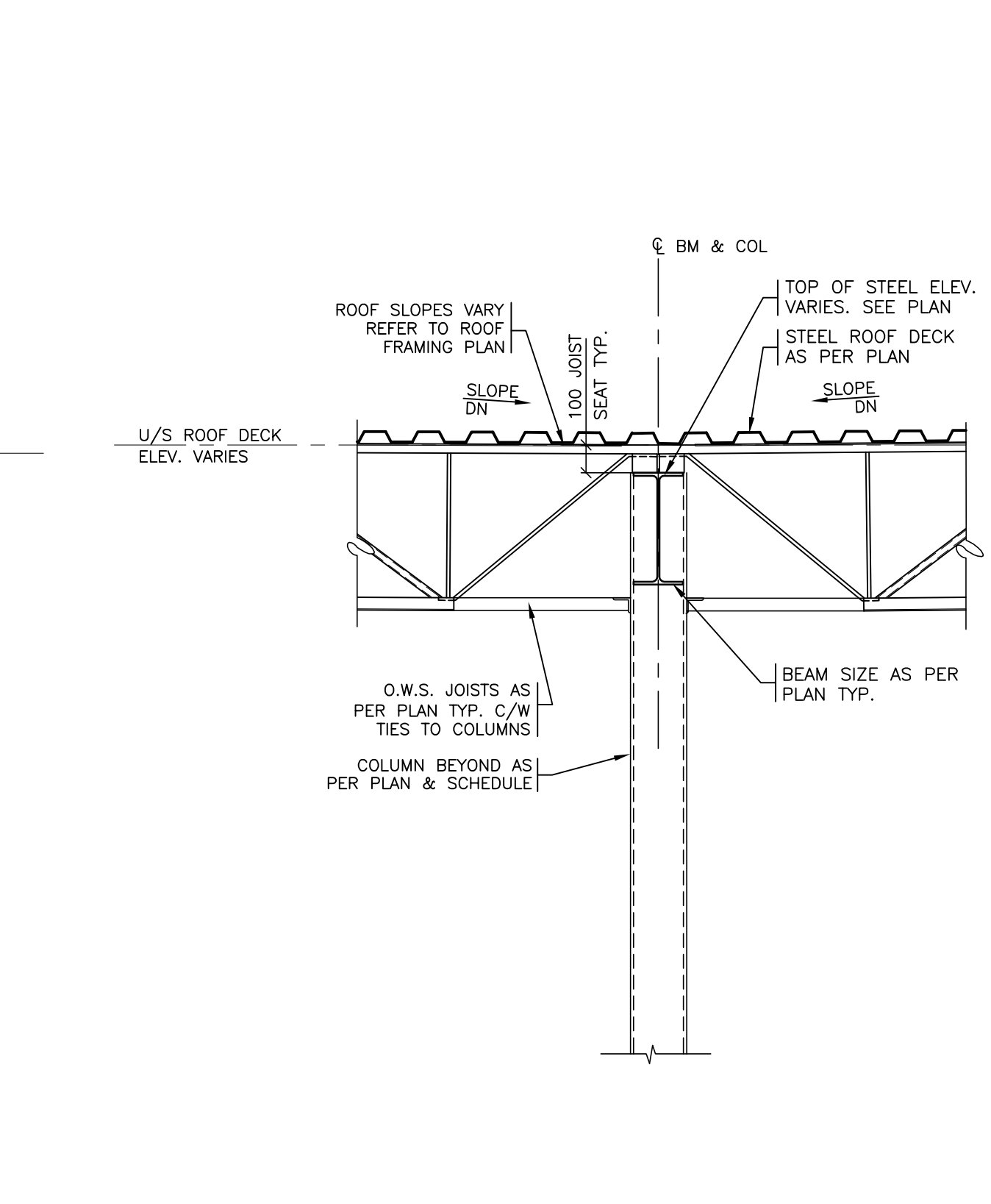
2 SECTION AT O/H DOOR OPN'G
 S5.1 SCALE 1:20



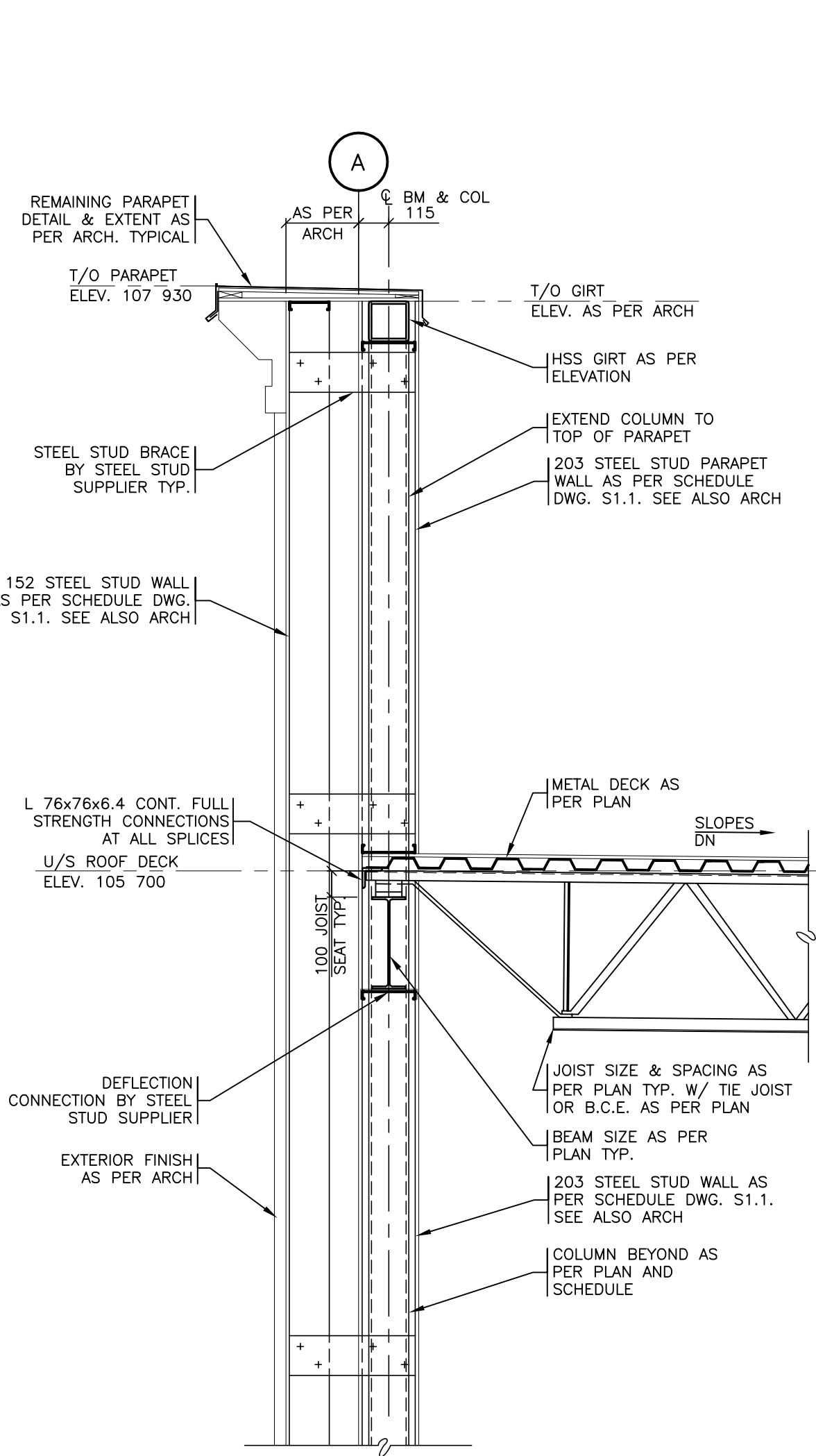
3 SECTION
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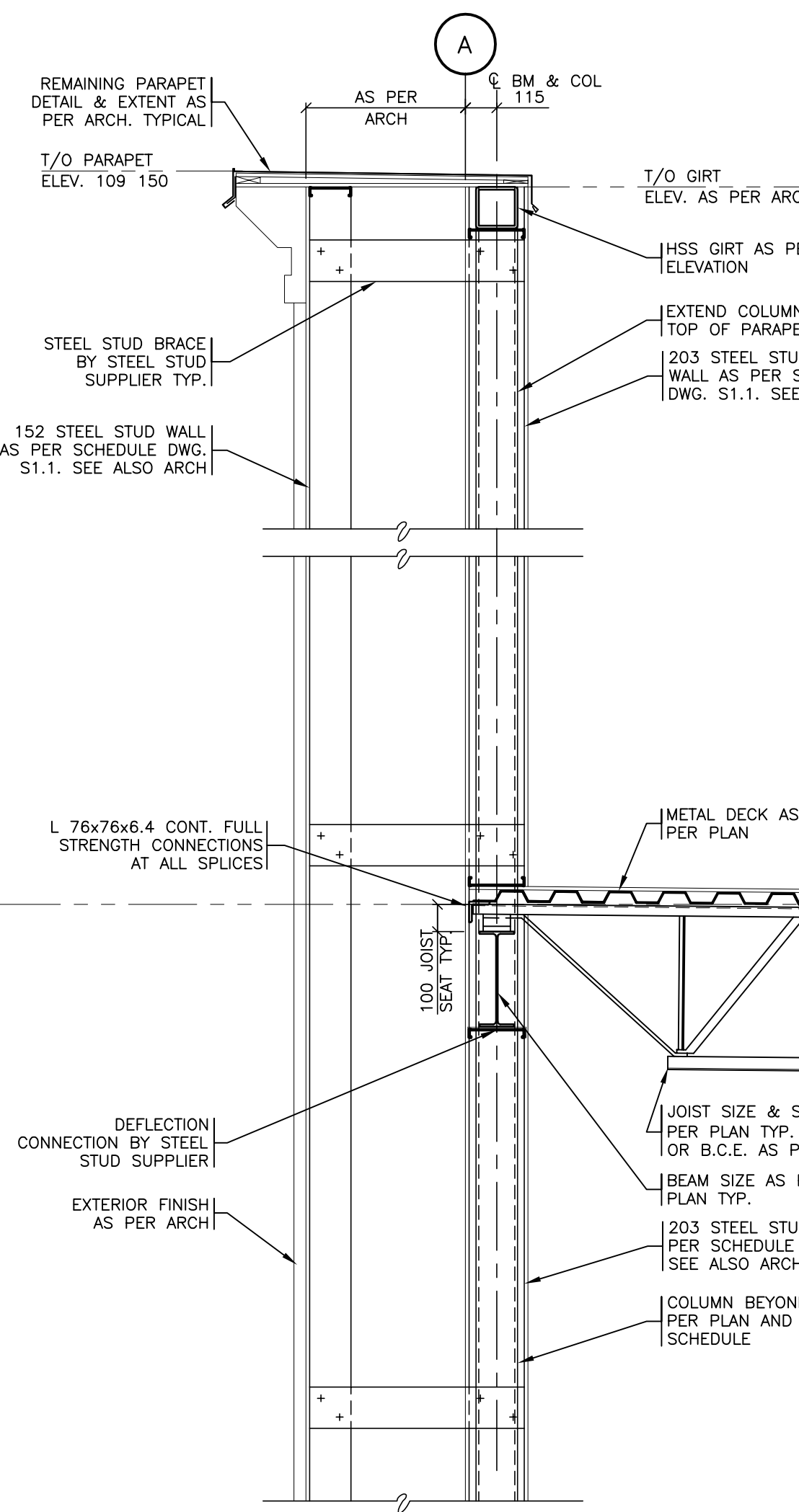
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 S5.1 SCALE 1:20



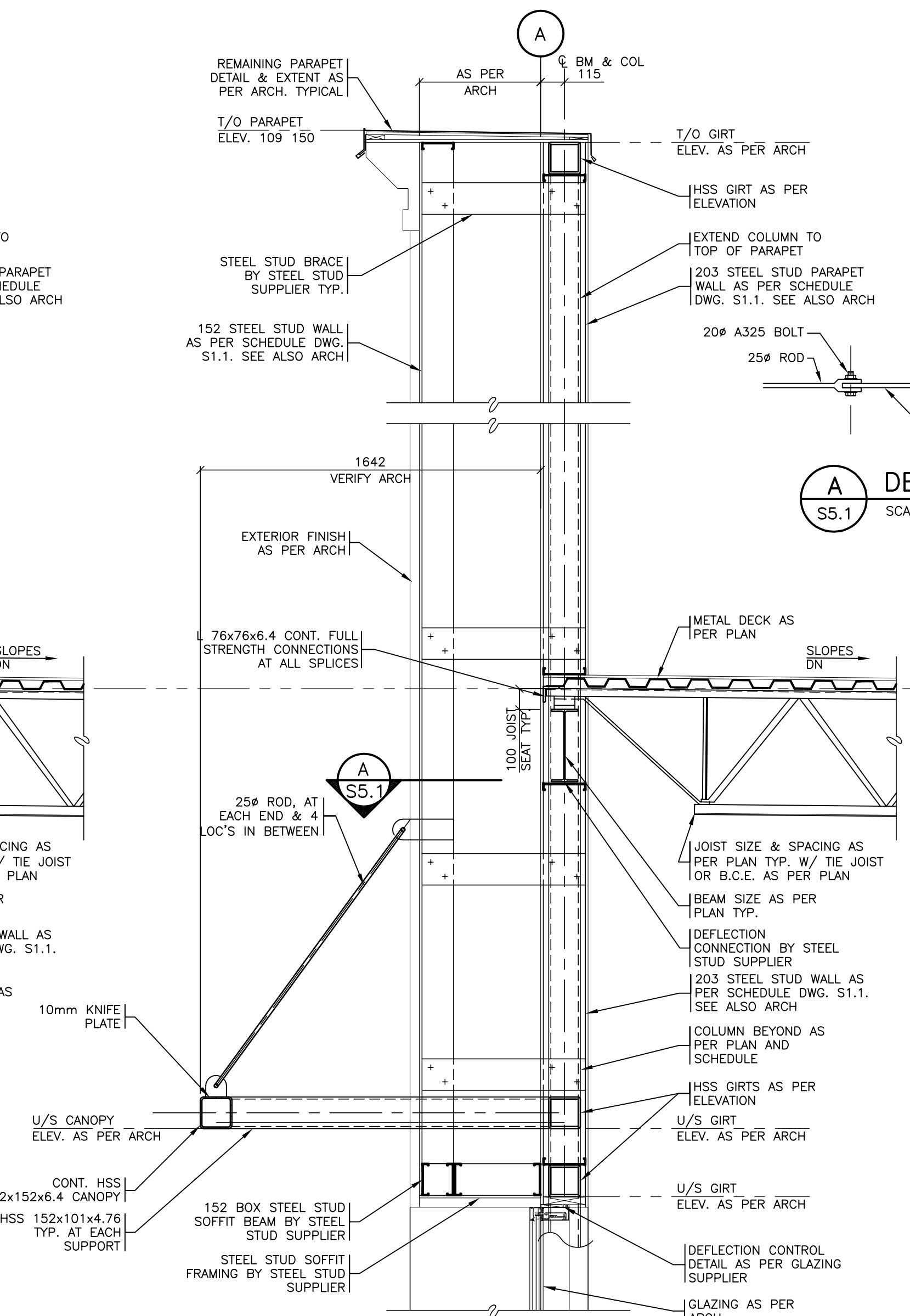
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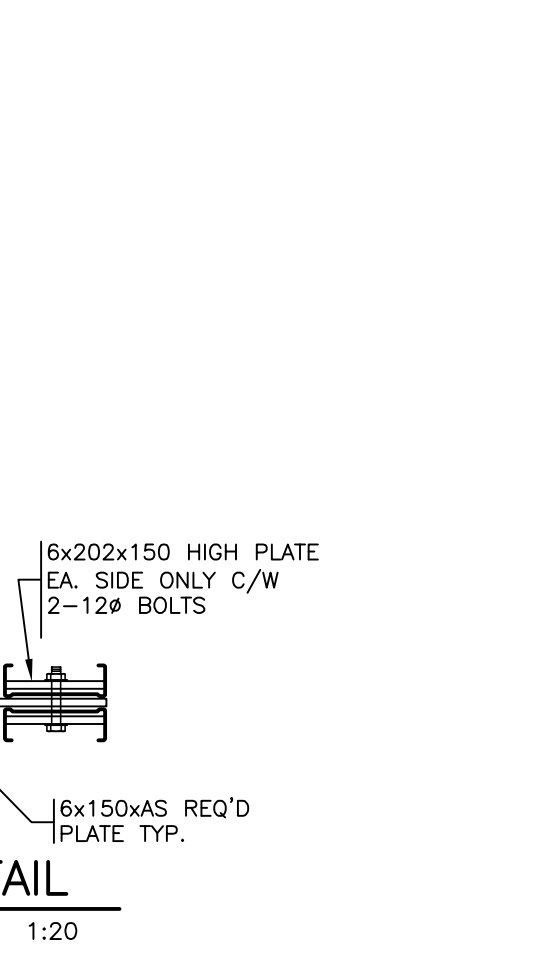
6 SECTION
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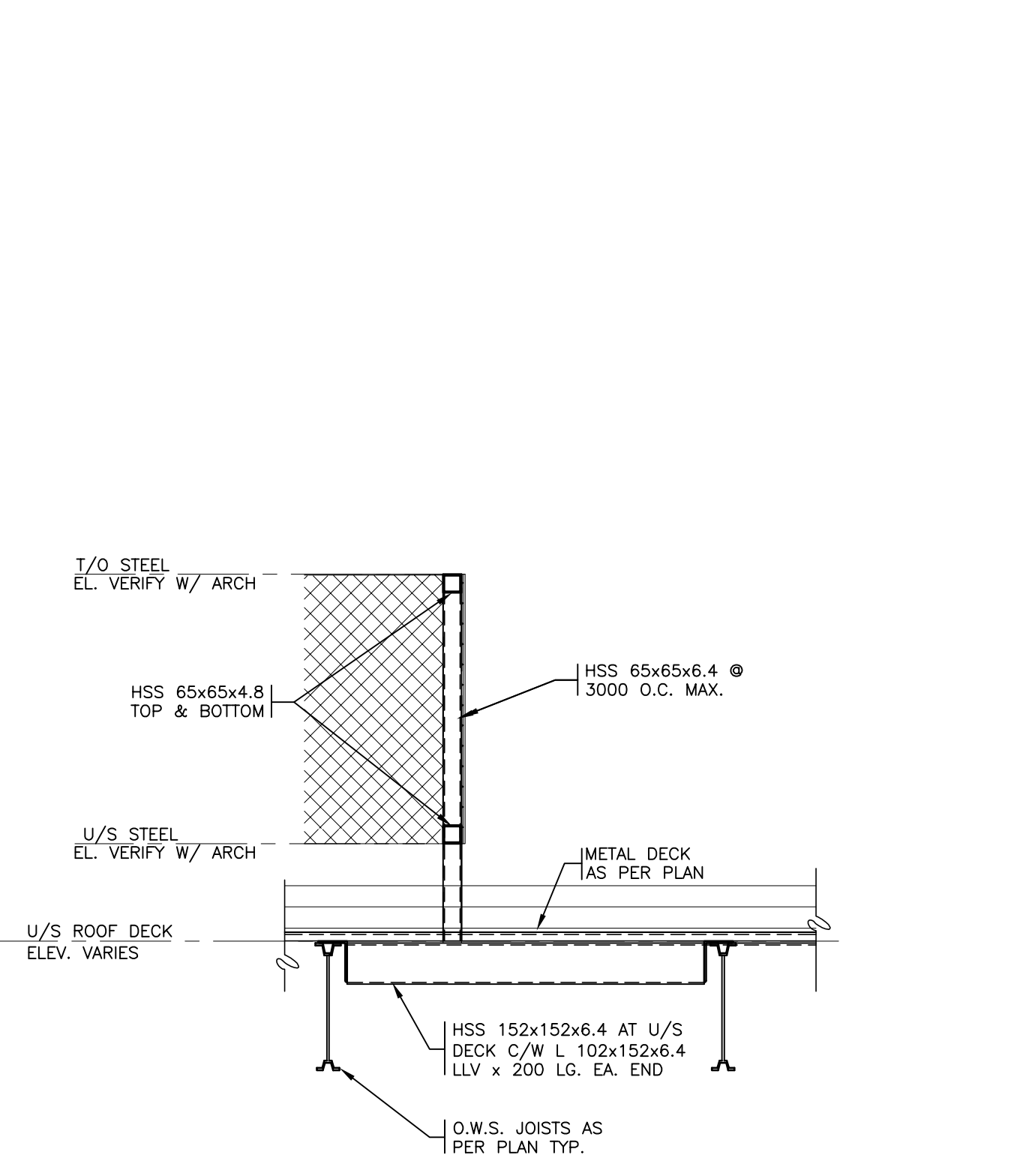
7 SECTION
 S5.1 SCALE 1:20



8 SECTION AT STOREFRONT
 S5.1 SCALE 1:20



A DETAIL
 S5.1 SCALE 1:20



9 SECTION AT MECH. SCREEN
 S5.1 SCALE 1:20

4	RE-ISSUED FOR TENDER	JB	13-02-20
3	ISSUED FOR PILING TENDER	PP	13-01-18
2	ISSUED FOR TENDER	PP	12-10-09
1	ISSUED FOR BUILDING PERMIT	WD	12-09-12
No.	Issued For	BY	YY-MM-DD Date

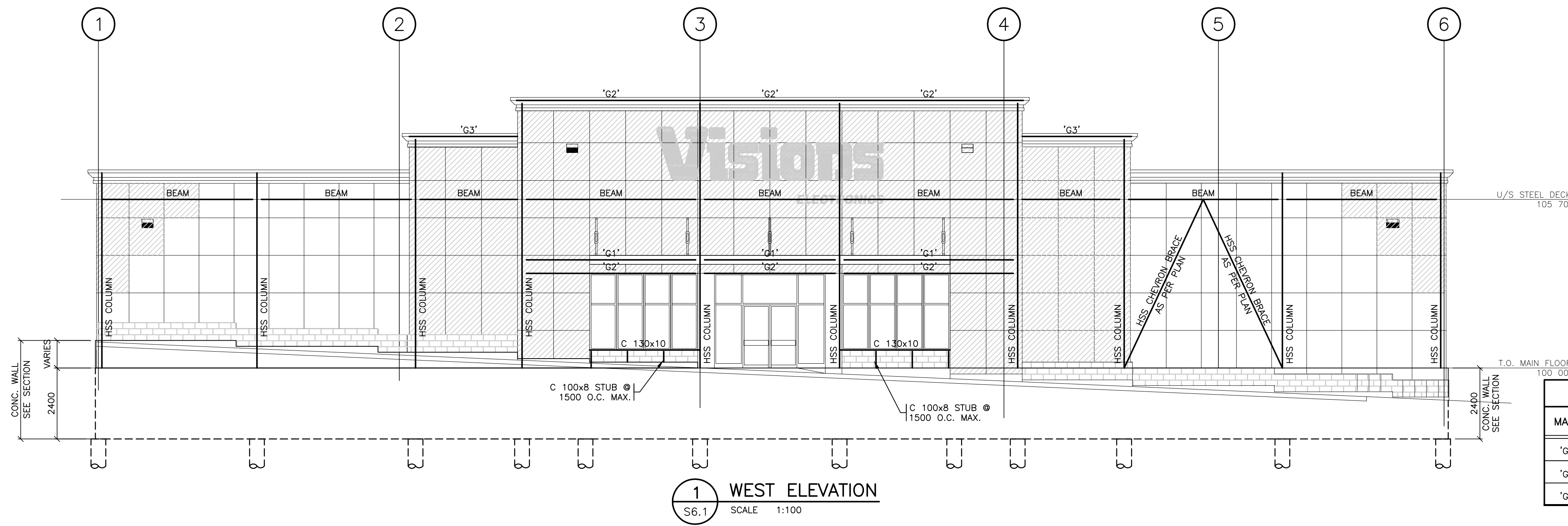
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architecture
 urban design
 engineering
 interior design

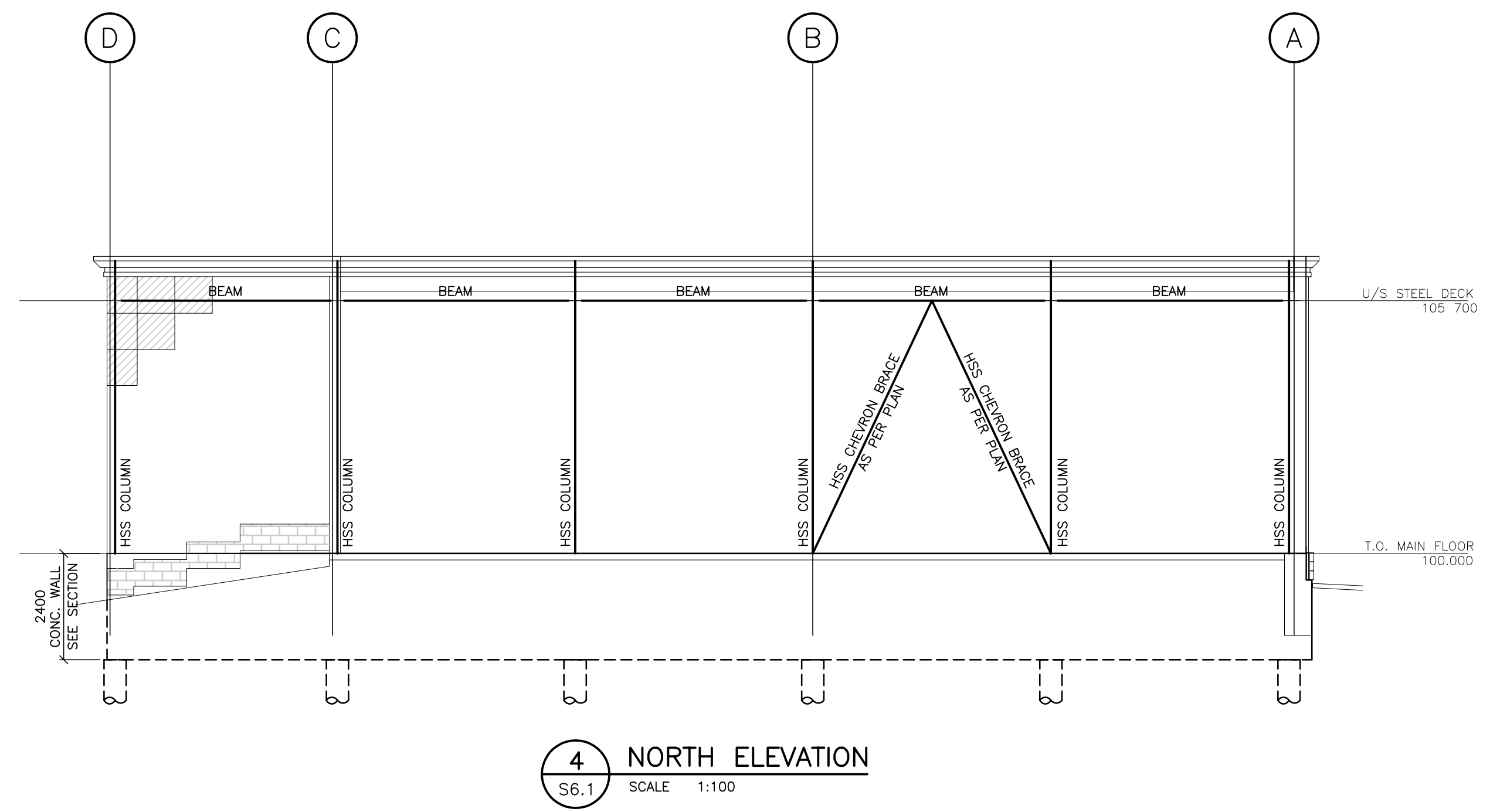
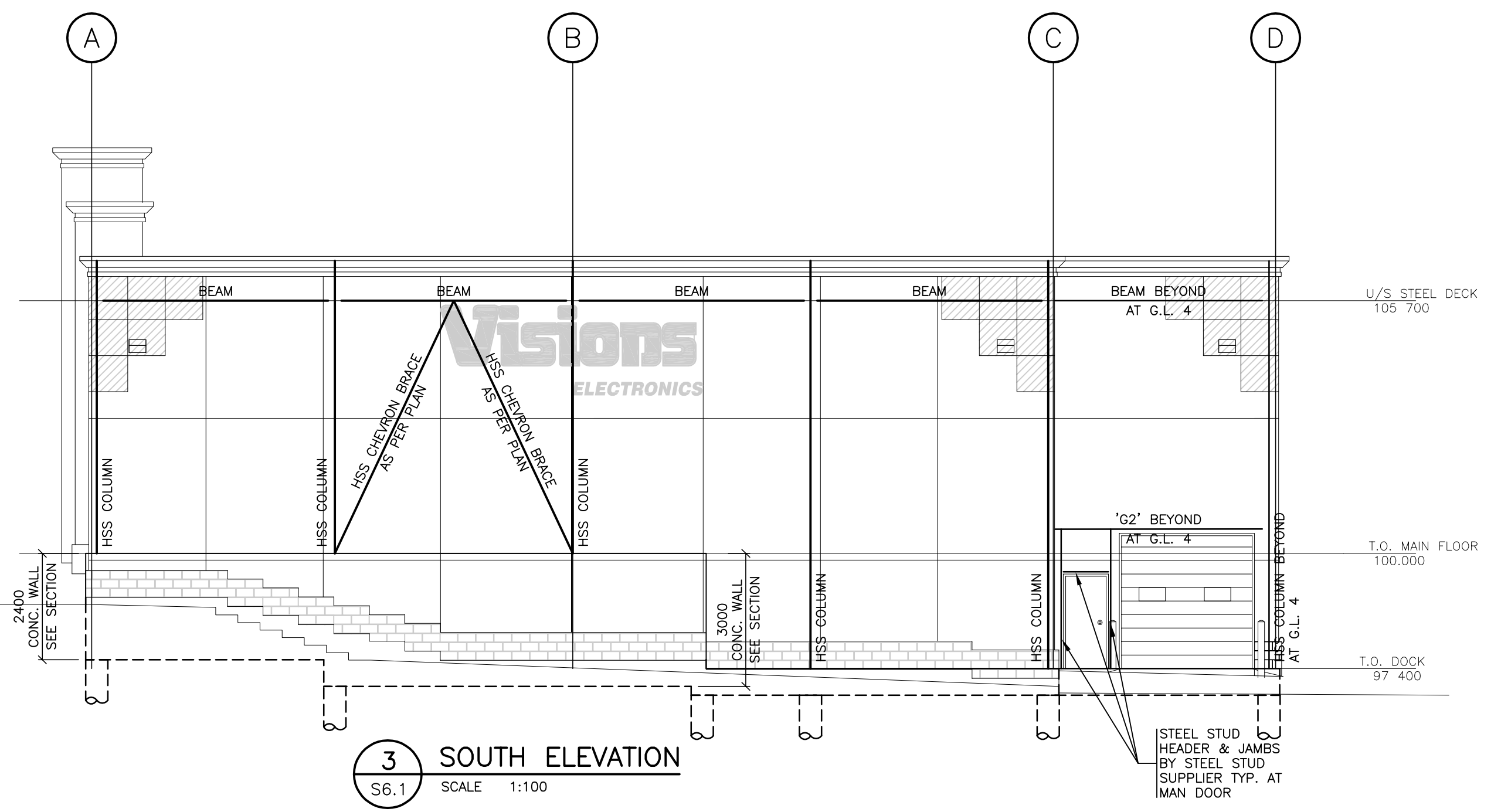
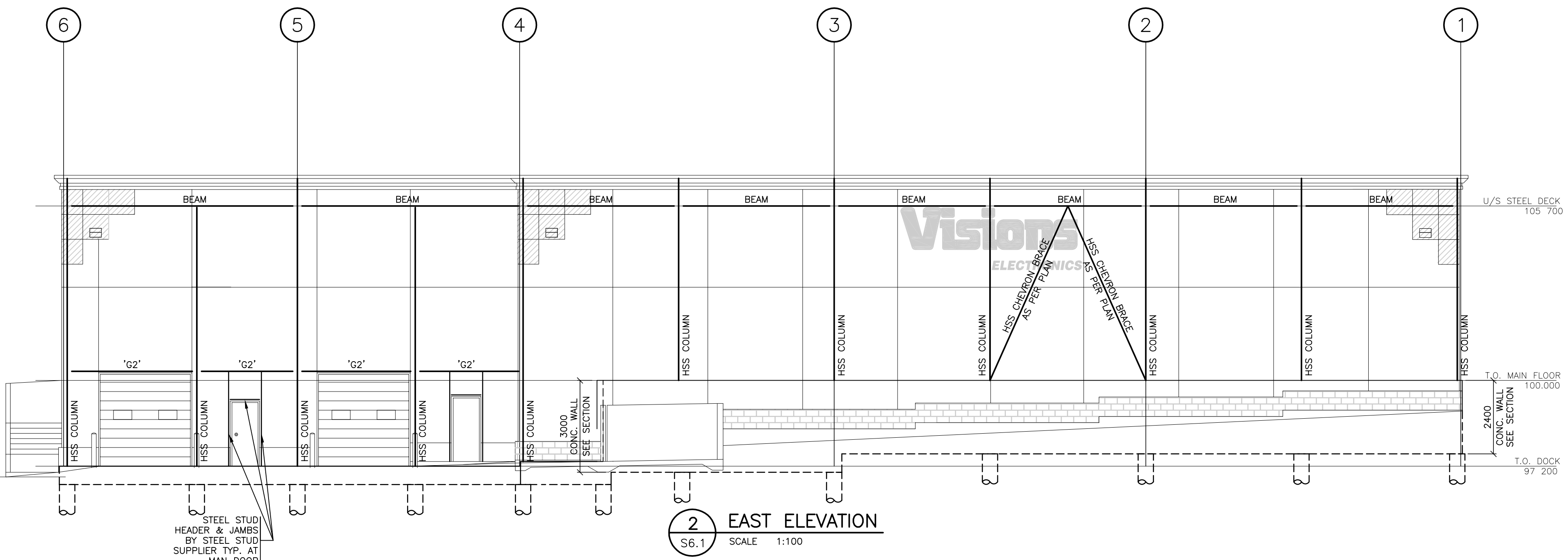
ST. ALBERT Commercial Development
 St. Albert Alberta

VISIONS ROOF SECTIONS

S5.1
 2012-092



HSS GIRT SCHEDULE	
MARK	DESCRIPTION
'G1'	HSS 203 x 203 x 13.0
'G2'	HSS 152 x 152 x 6.4
'G3'	HSS 152 x 152 x 4.8



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4	RE-ISSUED FOR TENDER	J8	13-02-20
3	ISSUED FOR PILING TENDER	PP	13-01-18
2	ISSUED FOR TENDER	PP	12-10-09
1	ISSUED FOR BUILDING PERMIT	VG	12-09-12
No.	Issued For	HR	YY-MM-DD Date

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ST. ALBERT
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VISIONS
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