- 1. STRUCTURAL DRAWINGS ARE TO BE READ IN CONJUNCTION WITH THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS ETC., FOR DETAILED DIMENSIONS OF DOORS, WINDOWS AND DUCT OPENINGS, REBATES, CHASES, NAILERS, ETC.
- 2. THE CONTRACTOR SHALL COMPARE ALL STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS AND SHALL CHECK AND VERIFY ALL DIMENSIONS BEFORE COMMENCING WITH THE WORK. ANY DISCREPANCIES NOT REPORTED TO THE ARCHITECT FOR CLARIFICATION WILL BECOME THE RESPONSIBILITY OF THE CONTRACTOR.
- 3. ATTACHMENTS FOR MECHANICAL, ELECTRICAL AND OTHER SERVICES SHALL BE MADE BY USING APPROVED CLAMPING DEVICES OR U-BOLT TYPE CONNECTORS. NO DRILLING OR CUTTING SHALL BE DONE UNLESS APPROVED BY THE ENGINEER.
- 4. THE CONTRACTOR SHALL RETAIN A PROFESSIONAL ENGINEER FOR THE DESIGN AND INSPECTION OF SHORING AND CONCRETE FORM WORK AS PER W.C.B. REGULATION 3428. UNDERPIN WHERE NECESSARY ANY EXISTING STRUCTURE AND PROVIDE ALL BRACING, SHORING, ETC., TO SUPPORT ADJOINING SOIL, FLOORS, WALLS, ETC., AS REQUIRED TO RETAIN ALL WORK IN PLACE AND PREVENT TEMPORARY OVER STRESSING OF THE
- 5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ADEQUATELY PROVIDING THE COMPONENTS THAT MAY BE NECESSARY FOR CONSTRUCTION SAFETY AND THE DESIGN AND ERECTION OF ALL TEMPORARY STRUCTURES.
- 6. THESE DRAWINGS CAN BE USED FOR CONSTRUCTION ONLY IF "ISSUED FOR CONSTRUCTION" IS MARKED IN THE REVISION COLUMN.

7. DESIGN LOADS

	LIVE LOAD	SUPERIMPOSED LOAD
-ROOF SNOW	45 PSF	INCLUDES RAIN COMPONENT
-MAIN FLOOR	100 PSF	
-SEISMIC AND WIND FORCES	THE NEW MEMBER	RS HAVE BEEN DESIGNED FOR

WIND AND SEISMIC LOADING IN ACCORDANCE WITH PART 4 OF THE 2012 BCBC THE SEISMIC CAPACITY OF THE ORIGINAL BUILDING HAS NOT BEEN DIMINISHED FROM IT'S ORIGINAL DESIGN CAPACITY

Se=2. 4 KPa Sr=0. 2 KPa q50=0. 44 KPa Sa(\(\Omega_2\))=1. | Sa(\(\Omega_5\))=\(\Omega_1\) | Sa(\(\Omega_0\))=\(\Omega_1\) | Sa(\(\Omega_0\))=\(\Omega_1\) | RGa = \(\Omega_1\) | RGa

- 8. THE CONTRACTOR SHALL ENSURE THAT THE CONSTRUCTION LOADS MUST NOT EXCEED THE ABOVE DESIGN LOADS.
- 9. SEE THE ARCHITECTURAL DRAWINGS AND SPECIFICATIONS FOR FIRE RATINGS AND LOCATIONS.
- 10. PRIOR TO POURING ANY CONCRETE OR OTHERWISE CONCEALING ANY STRUCTURAL ELEMENTS, THE CONTRACTOR SHALL NOTIFY LONDON MAH AND ASSOCIATES AND GIVE ADEQUATE ACCESS AND REASONABLE TIME IN ORDER TO INSPECT THE CONDITIONS OF THE WORK.

FIELD REVIEW

LONDON MAH & ASSOCIATES (LMA) PROVIDES FIELD REVIEW ONLY FOR THE WORK SHOWN ON THE STRUCTURAL DRAWINGS PREPARED BY LMA. THIS REVIEW IS A PERIODIC REVIEW AT THE PROFESSIONAL JUDGEMENT OF LMA. THE PURPOSE OF THIS REVIEW IS TO ASCERTAIN THAT THE WORK IS IN GENERAL CONFORMANCE WITH THE PLANS AND SUPPORTING DOCUMENTS PREPARED BY LMA AND TO FACILITATE COMPLETION OF THE LETTERS OF ASSURANCE REQUIRED BY BUILDING CODE REQUIREMENTS. FIELD REVIEW BY LMA IS NOT CARRIED OUT FOR THE CONTRACTOR'S BENEFIT, NOR DOES IT MAKE LMA GUARANTORS OF THE CONTRACTOR'S WORK. IT REMAINS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE HIS OWN QUALITY CONTROL AND TO PERFORM THE WORK WITH GOOD WORKMANSHIP AND IN CONFORMANCE WITH THE CONTRACT

THE CONTRACTOR MUST PROVIDE SAFE ACCESS TO ALL AREAS REQUIRING INSPECTION IN ACCORDANCE WITH THE CURRENT WCB OCCUPATIONAL HEALTH &

SITE PREPARATION

THE CONTRACTOR SHALL PERUSE THE GEOTECHNICAL REPORT IN CONJUNCTION WITH THE SITE PLANS IN ORDER TO ASCERTAIN THE WORK REQUIRED. CONSULT THE GEOTECHNICAL ENGINEER AS TO BACKFILLING MATERIALS ETC.

FOUNDATIONS

- . THE FOUNDATIONS HAVE BEEN DESIGNED FOR THE FOLLOWING MAXIMUM ALLOWABLE BEARING PRESSURES. (GEOPACIFIC REPORT DATED MARCH 8, 2017) FOUNDATION 2500 PSF
- 2. BEARING SURFACES MUST BE INSPECTED AND CONFIRMED BY A GEOLOGICAL ENGINEER PRIOR TO POURING FOOTING CONCRETE.

CONCRETE AND REINFORCING

I. REFERENCE STANDARDS CSA-A23.1-Ø4. CSA-A232-Ø4. CSA-A23.3-Ø4. G30.5-M1983(R1991). G3Ø.15-M1983(R1991). CAN/CSA-G3Ø.18-M92. ASTM ATTS. CSA S269,1-1975.

CAN/CSA 5269.3-M92.

ACI 347.

CONC. MATERIALS AND METHOD OF CONC. CONST'N METHODS OF TEST FOR CONCRETE DESIGN OF CONCRETE STRUCTURES WELDED STEEL WIRE FABRIC FOR CONC. REINF'T WELDED DEF'D STEEL WIRE FABRIC FOR CONC. REINF'T BILLET-STEEL BARS FOR CONC. REINFOR'T. EPOXY COATED REINFORCING BARS MANUAL FOR STD PRACTICE FOR DETAILING. FALSEWORK FOR CONSTRUCTION PURPOSES

RECOMMENDED PRACTICE FOR CONC. FORMUK

2. ALL CONCRETE WORK AND TESTING SHALL CONFORM TO THE REQUIREMENTS OF APPROPRIATE STANDARDS. CONCRETE DENSITY SHALL BE 2350 KG/M3 (150 P.C.F.). CEMENT SHALL BE PORTLAND CEMENT TYPE 10, UNLESS NOTED OTHERWISE. CONCRETE FOR VARIOUS PURPOSES SHALL BE AS FOLLOWS:

28 DAY CLASS MAX. COM-OF SIZE PRESSIVE EXPO- AGGRE- SLUMP AIR STRENGTH SURE GATE (mm) (%) FOUNDATION AND INT. SLAB ON GRADE 25 MPa 2Ø mm 80±10 EXTERIOR SLAB ON GRADE 40 mm 80±10 4-7 C2

3. REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF C.S.A. G30.18M AND SHALL BE NEW BILLET STOCK OF THE FOLLOWING STRENGTHS:

SMALLER THAN 10M MINIMUM YIELD POINT: 300 MPa IOM AND LARGER MINIMUM YIELD POINT: 400 MPa

CONCRETE FORMWORK.

- CONTRACTOR SHALL ARRANGE FOR THE TAKING AND THE TESTING OF 100 x 200 mm CONCRETE CYLINDERS BY AN INDEPENDENT TESTING LABORATORY APPOINTED BY THE OWNER COPIES OF ALL CONCRETE TEST REPORTS TO BE SENT TO LONDON MAH AND ASSOCIATES LTD.
- 5. ALL REINFORCING BARS SHALL BE TIED SECURELY TO PREVENT DISPLACEMENT. WALL REINFORCING, INCLUDING DOWELS, TO LAP 40 BAR DIAMETERS WITH 90° HOOKS AT CORNERS OR BENT BARS OF SAME SIZE AND SPACING AS HORIZONTAL REINFORCING. ENDS OF WALLS TO HAVE TWO 15M VERTICAL (MIN.). UNLESS OTHERWISE NOTED, REINFORCEMENT TO BE:

10M @ 17" VERT. 10M @ 13" VERT. 8" WALLS 10M @ 8" HORIZ.

6. BAR DESIGNATION CODE

8-25M x 10'-6" MEANS 8 - 25M x 10'-6" LONG TOP BARS BOT. BARS ___ ___ ___ MEANS HOOKED ONE END MEANS HOOKED TWO ENDS MEANS UNLESS NOTED OTHERWISE MEANS EACH WAY MEANS INSIDE FACE MEANS OUTSIDE FACE MEANS EACH FACE

ALL REINFORCING SHALL BE FREE AND CLEAN OF ALL SUBSTANCES THAT WILL PREVENT PROPER BOND. OIL FORMS PRIOR TO THE PLACEMENT OF ALL

7. CONCRETE COVER TO REINFORCING AS FOLLOWS:

SURFACES PLACED IN CONTACT WITH GROUND FORMED SURFACES EXPOSED TO GND OR WEATHER

8. <u>EMBEDMENT OF DOWELS</u>

	<u>COMPRESSION</u>			<u>TENSION</u>		
BAR	25	3Ø+		25	3Ø	35
	<u>MPa</u>	<u>MPa</u>		<u>MPa</u>	<u>MPa</u>	<u>MPa</u>
IØM	9"	8"		13"	12"	12"
15M	12"	12"		19"	17"	16"
	15"	14"		23"	21"	19"
	19"	18"		36"	33"	32"
	23"	21"		43"	37"	36"
	27"	25"		51"	47"	43"
			MAY IN	N EMBEDI NCLUDE 90 PATION DE	0° HOOK	

WHERE EMBEDMENT DIMENSIONS ARE CALLED FOR, SUCH DIMENSIONS SHALL

INSUFFICIENT FOR STRAIGHT

- SPLICES FOR REINFORCEMENT
- WELDED OR MECHANICAL SPLICES MAY BE USED FOR ALL BARS PROVIDED THAT: SPLICE ALTERNATE BARS TO A MAXIMUM OF 1/2 THE TOTAL BAR GROUP AT ANY ONE POINT. THE MINIMUM DISTANCE BETWEEN SPLICES ON ALTERNATE BARS TO BE 40 BAR DIAMETERS.
- B. THE SPLICE DEVELOPS 100% OF THE COMPRESSIVE STRENGTH OF THE BAR BAR AND 150% OF THE TENSILE STRENGTH.
- C. TEST RESULTS OF THE SPLICE ARE TO BE PROVIDED FOR THE

ENGINEER'S APPROVAL BY THE CONTRACTOR. ALL LAPPED SPLICES TO BE AS PER THE FOLLOWING UNLESS OTHERWISE SHOWN ON THE DRAWINGS:

LAP LENGTH IN INCHES

BAR	COMPRESSION	CLASS "B	" TENSION	<u>SPLICE</u>	
<u>SIZE</u>	SPLICE	25	30	35	
		<u>MPa</u>	<u>MPa</u>	<u>MPa</u>	
10M		ורן"	16"	14"	
15M	18"	22"	20"	19"	
20M	24"	30"	27"	25"	
25M	3Ø"	46"	43"	39"	
30M	35"	56"	51"	47"	
35M	41"	65"	60"	55"	

FOR TENSION SPLICES IN MEMBERS OTHER THAN SHEAR WALLS, SPLICE NO MORE THAN 50% OF THE BARS AT ANY ONE LOCATION

- 10. REINFORCING AT OPENINGS SHALL NOT BE CUT OR BENT, BUT SHALL BE FANNED WHERE POSSIBLE OR CROWDED EITHER SIDE TO CLEAR OPENINGS.
- ALL EXPOSED CORNERS OF CAST-IN-PLACE CONCRETE TO HAVE 34 x 34
- CHAMFERS UNLESS OTHERWISE NOTED.

WHERE CONCRETE SURFACES ARE TO BE EXPOSED, ONLY NON-CORROSIVE TYPE

- REINFORCING CHAIRS SHALL BE USED TO SUPPORT REINFORCING.
- ALL VERTICAL CONSTRUCTION JOINTS BELOW GRADE SHALL HAVE 6" PVC WATER STOPS.
- 14. ALL SUMPS, PITS, TRENCHES, ETC., TO HAVE 6" WALLS AND BOTTOM. REINFORCE WITH 10M @ 16" EACH WAY UNLESS OTHERWISE NOTED. WHERE CONDUITS ARE EMBEDDED IN SLABS:
- A. CONDUIT SHALL BE KEPT 12" AWAY FROM WALLS AND COLUMNS. SLAB THICKNESS SHALL BE INCREASED I" IN AREAS WHERE CLEAR
- DISTANCE BETWEEN CONDUITS IS LESS THAN 2 CONDUIT DIAMETERS.
- C. IN NO CASE SHALL THE CLEAR DISTANCE BETWEEN THE CONDUITS BE LESS THAN I" WITHOUT PRIOR APPROVAL OF THE ENGINEER.
- 15. NO CALCIUM CHLORIDE IS TO BE USED WITHOUT THE WRITTEN PERMISSION OF THE ENGINEER.
- 6. NOTIFY THE ENGINEER AT LEAST 48 HOURS IN ADVANCE OF PLACING CONCRETE TO INSPECT REINFORCING.
- GENERAL CONTRACTOR TO SUPPLY A CLOSED BOX WITH LID TO STORE CONCRETE TEST CYLINDERS FOR A MINIMUM OF 24 HOURS AFTER FILLING. THE CYLINDERS WITH THE CONCRETE TO BE TESTED.

18. <u>COLD WEATHER REQUIREMENTS:</u>

- FORECASTED AIR TEMPERATURE NOT BELOW 2° C: A. IF CONCRETE TEMPERATURE DROPS BELOW 10° C AT POINT OF
- POURING, THE MIXING WATER SHALL BE HEATED TO MAINTAIN A MINIMUM CONCRETE TEMPERATURE OF 10° C.
- CONCRETE SHALL NOT BE PLACED ON OR AGAINST ANY SURFACE WHICH IS AT A TEMPERATURE LESS THAN 4° C.
- THE CONTRACTOR SHALL BE PREPARED TO COVER THE SLAB IF UNEXPECTED DROP IN AIR TEMPERATURE SHOULD OCCUR.
- 2. FORECASTED AIR TEMPERATURE BELOW 2° C BUT NOT BELOW -4° C: A. FORMS AND STEEL SHALL BE FREE FROM ICE AND SNOW.
- B. MIXING WATER SHALL BE HEATED TO GIVE A MINIMUM CONCRETE
- TEMPERATURE OF 10° C AT POINT OF POUR C. CONCRETE SHALL BE HEATED TO GIVE A MINIMUM CONCRETE
- TEMPERATURE OF 10°C AT POINT OF POUR D. SLABS SHALL BE COVERED WITH CANVAS OR SIMILAR MATERIAL AND KEPT 4" MINIMUM CLEAR OF SURFACE FOR AIR
- CIRCULATION. E. PROTECTION SHALL BE MAINTAINED FOR AT LEAST THREE DAYS.
- 3. FORECASTED AIR TEMPERATURE BELOW -4° C: A., B., C. AND D. AS UNDER POINT 2.
- E. STOREY BELOW SHALL BE ENCLOSED AND ARTIFICIAL HEAT PROVIDED. HEATING TO BE STARTED AT LEAST ONE HOUR AHEAD OF POURING AND MAINTAINED FOR A MINIMUM OF THREE DAYS
- TEMPERATURE OF THE CONCRETE AT ALL SURFACES SHALL BE KEPT AT A MINIMUM OF 20° C FOR THREE DAYS OR 10° C FOR FIVE DAYS, CONCRETE SHALL BE KEPT ABOVE FREEZING TEMPERATURE FOR AT LEAST SEVEN DAYS.
- G. ENCLOSURE MUST BE CONSTRUCTED SO THAT AIR CAN CIRCULATE OUTSIDE THE OUTER EDGES AND MEMBERS.

1-1/2"

- ALL SAWN LUMBER HAS BEEN DESIGNED TO CSA-086-01 AND THE CONTRACTOR MUST ENSURE THAT ALL MATERIALS COMPLY WITH THE MATERIAL STANDARDS REFERENCED IN THIS STANDARD.
- FRAMING LUMBER SHALL BE WELL SEASONED AND TO BE OF THE FOLLOWING GRADES UNLESS NOTIFIED OTHERWISE:

A.	STUDS
B.	BUILT-UP BEAMS
C.	LINTELS & HEADERS
D.	TOP & BOT. PLATES
E.	FREE STANDING COLS

SPF STUD GRADE OR BETTER (KILNDRIED) SEE BEAM SCHEDULE SEE BEAM SCHEDULE DFIR NO.3 OR BETTER (KILNDRIED) D.FIR NO.2 OR BETTER (KILNDRIED)

FLAT ROOF SHEATHING EXTERIOR WALLS

%" SELECT T & G FIR PLYWOOD 1/2" D. FIR WALL PLY. ALL PANELS TO BE LAID STAGGERED. FLOOR AND ROOF PANELS TO BE FASTENED TO SUPPORTS WITH $2last_2$ " COMMON NAILS AT 4" O.C. ALONG PANEL EDGES AND AT 12" O.C. ALONG INTERMEDIATE SUPPORTS. FOR WALI SHEATHING EDGE NAILING, SEE PLANS FOR NAILS AND NAILING PATTERNS (PROVIDE NAILS AT 12" O.C. ALONG INTERMEDIATE SUPPORTS UNLESS NOTED OTHERWISE). PROVIDE SOLID BLOCKING ALONG SIDES OF EACH SHEET.

- STUDS SUPPORTING BEAMS SHALL HAVE AT LEAST THE SAME NUMBER AND/OR SAME WIDTH AS THE BUILT-UP BEAMS OR SOLID BEAMS.
- 5. APPROVED JOIST HANGERS TO BE USED AT FLUSH CONNECTIONS. APPROVED CONNECTORS SHALL BE USED FOR POST AND BEAM CONNECTIONS AND POST BASE CONNECTIONS
- 6. LINTELS AND HEADERS TO BE 2 2 x 10 WHEN NOTED, TYPICAL OVER DOORS WINDOWS AND OTHER OPENINGS ETC.
- 1. ALL COLUMNS, NOT BUILT INTO WALLS, SHALL BE 6×6 DFIR (NO.2 OR BETTER) UN.O
- 11. BUILT-UP STUDS WITHIN WALLS OR OTHERWISE SHALL BE NAILED AS FOLLOWS 2 x 4 STUDS 2 x 6 STUDS
- USE 3", 4" AND 6" NAILS FOR 2 PLY, 3PLY AND 4 PLY COLUMNS RESPECTIVELY
- 8. STUD BEARING WALL
- 2 x 6 @ 16" o.c. SEE ALSO ARCHITECTURAL DRAWINGS FOR WALL SCHEDULE INFO.
- 9. ALL DRILLED HOLES LARGER THAN $rac{1}{2}$ " DIAMETER IN JOISTS SHALL BE PRE-APPROVED BY THE ENGINEER
- 10. ALL SUPPORTING STUDS FOR BEAMS AND GIRDERS SHALL BE CARRIED DOWN TO THE FOUNDATION LEVEL
- II. ALL TOP AND BOTTOM PLATES SHALL BE DFIR NO.3 OR BETTER UN.O
- 12. EACH PLY OF BUILT-UP BEAMS (MAXIMUM 4 PLY UN.O) SHALL BE NAILED
- WITH 3 ROWS OF 31/2" NAILS 12" O.C.. 13. FOR ANCHORAGE OF WOOD FRAME STUD WALLS TO THE SUPPORTING CONCRETE
- STRUCTURE, USE DFIR NO.3 OR BETTER BASE PLATES WITH 5/8" DIA. KWIK BOLTS ALL ROOF DECK, INCLUDING DIAPHRAGM NAILING, TO BE INSTALLED PRIOR TO
- INSTALLING PARAPET FRAMING. SLOTS IN ROOF DECK TO BE CUT TO FEED AND INSTALL SIMPSON TENSION STRAPS.
- SIMPSON TENSION STRAPS AROUND THE PERIMETER OF THE BUILDING TO BE INSTALLED PRIOR TO PLYWOOD WALL SHEATHING.
- 16. BUILT UP WOOD POSTS, LINTELS AND BEAMS SHALL BE FASTENED TOGETHER WITH NAILS SPACED NOT MORE THAN 12" O/C OR 3/4" DIAMETER BOLTS FITTED WITH WASHERS AND SPACED NOT MORE THAN 18" O/C.

STRUCTURAL STEEL AND MISCELLANEOUS METALS

CSA S136,1-Ø1.

. REFERENCE STANDARDS CAN/CSA-SI6-ØI LIMIT STATES DESIGN OF STEEL STRUCTURES

CSA W59-Ø3. WELDED STEEL CONSTRUCTION CSA W47.1-Ø3. CERTIFICATION OF WELDERS CAN/CSA G4Ø21-Ø4 STRUCTURAL QUALITY STEELS SPECIFICATION FOR STRUCTURAL STEEL ASTM A36. ASTM A3Ø7.-ØØ BOLTS AND NUTS. ASTM A325M-Ø2. HIGH STRENGTH BOLTS.

- 2. NO FIELD WELDING IS PERMITTED EXCEPT AS SPECIFICALLY DETAILED OR OTHERWISE APPROVED BY THE ENGINEER. FABRICATION WILL BE RESTRICTED TO CERTIFIED WELDING FABRICATORS AND CONTRACTORS HAVING CERTIFICATION FROM THE CANADIAN WELDING BUREAU TO THE REQUIREMENTS OF C.S.A. W47.1-92. THE CONTRACTOR MUST ENSURE THAT HIS FABRICATORS HAVE THIS CERTIFICATION.
- 3. SUBMIT SHOP DRAWINGS AND COPIES OF THE FABRICATOR'S C.W.B. CERTIFICATES TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS FOR CONNECTIONS TO BE SIGNED AND SEALED BY A PROFESSIONAL
- ENGINEER REGISTERED IN BRITISH COLUMBIA. 4. MISCELLANEOUS METALS TO BE PROVIDED BY STRUCTURAL STEEL SUPPLIER

COLD FORMED STEEL STRUCTURAL MEMBERS.

- 5. ALL STRUCTURAL STEEL IS TO CONFORM TO CSA G4020-04/G4021-04, W SECTIONS TO BE 350W, ALL OTHER ROLLED SECTIONS TO BE 300W. RECTANGULAR TUBE COLUMNS TO BE CLASS C AND SHALL BE 350W MATERIAL. WELDING IS TO BE IN ACCORDANCE WITH C.S.A. W59 USING E-10 ELECTRODES.
- 6. ALL STRUCTURAL STEEL, EXCEPT GALVANIZED SURFACES, IS TO BE PAINTED WITH ONE SHOP COAT OF PRIMER CONFORMING TO CISC/CPMA STANDARD I-
- 1. ALL BOLTS EXCEPT ANCHOR BOLTS ARE TO CONFORM TO A.S.T.M. A325, TYPE I, AND HAVE BEEN DESIGNED FOR BEARING-TYPE CONNECTIONS WITH THREADS EXCLUDED FROM THE SHEAR PLANE. ALL ANCHOR BOLTS TO CONFORM TO ASTM A36 EXCEPT WHERE OTHER NOTED ALL BOLTS TO BE 3/4" EXCEPT WHERE OTHER NOTED. ALL FILLET WELDS TO HAVE A 3/16" THROAT SIZE UNLESS OTHERWISE NOTED
- 8. BOLTED CONNECTIONS SHALL BE TORQUE-TESTED IN ACCORDANCE WITH CAN/CSA-
- 9. REINFORCING FOR OPENINGS IN STEEL DECK LARGER THAN 18 INCH DIAMETER OR 18 INCH ON A SIDE IS TO BE DESIGNED, DETAILED, SUPPLIED AND
- INSTALLED BY THE STRUCTURAL STEEL SUPPLIER UNLESS OTHERWISE SHOWN. 10. ALL STEEL WORK SHALL BE ERECTED TRUE AND PLUMB AND TEMPORARY BRACING SHALL BE INTRODUCED WHEN EVER NECESSARY TO TAKE CARE OF ALL LOADS TO WHICH THE STRUCTURE MAY BE SUBJECTED, INCLUDING ERECTION EQUIPMENT AND THE OPERATING OF SAME. SUCH BRACING SHALL BE LEFT IN
- 11. ALL STEEL MEMBERS, ELEMENTS, BOLTS, AND WASHERS EXPOSED PERMANENTLY TO THE ELEMENTS SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION IN ACCORDANCE WITH CSA-G164. ALL FIELD DAMAGE OR FIELD WELDING IS TO BE TOUCHED UP WITH TWO LIBERAL COATS OF "GALVICON"

BLDG. D

LIQUOR STORE

PLACE FOR AS LONG AS MAY BE REQUIRED FOR SAFETY

COLD-FORMED WIND BEARING STEEL STUDS: (15/8 " MIN. FLANGE)

- I. ALL STUDS SHALL BE OF TYPE SIZE AND GAUGE SHOWN ON DRAWINGS. ALL 18 GAUGE OR LIGHTER STUDS SHALL BE FORMED FROM STEEL WITH MINIMUM YIELD OF 33,000 PSI. FOR 16 GAUGE OR THICKER STUDS SHALL BE FORMED FROM STEEL WITH YIELD STRENGTH OF 50,000 PSI MIN.
- THE THICKNESS RELATIONSHIP BETWEEN GAUGE AND INCH'S PER BELOW(UN.O): 14GA=0.68- .075" 12GA=.090-0.97"
- ALL SCREWS SHOWN ON DRAWINGS ARE HILTI KWIK-PRO SELF-DRILLING SCREWS OR EQUIVALENT UN.O i.e *8 MEANS *8-18 ,
- #10 MEANS #10-16, *12 MEANS *12-14 FOR STUDS CONN. OR #12-24 FOR STUD/TRACK TO CONC/STRUC STEEL CONN. S.P. MEANS HILTI X-DNI/ X-EDNI FASTENER
- SEE DRAWING FOR NUMBER REQUIRE UN.O 2. ALL STUD COMPONENTS SHALL BE L.Z.C. GALVANIZED.
- 3. STEEL FRAMING COMPONENTS SHALL BE CUT SQUARELY OR AS REQUIRED TO FIT NEATLY AGAINST ABUTTING MEMBER. JOINING OF MEMBERS SHALL BE DONE BY FILLET, PLUG, BUTT OR SEAM WELDING OR BY SELF-DRILLING, SELF-TAPPING METAL SCREWS. CUTTING OF MEMBERS MAY BE SAW OR SHEAR. TORCH CUTTING IS NOT PERMITTED.
- 4. ALL STUDS SHALL BE REINFORCED AS SHOWN AND ERECTED TRUE AND PLUMB AND TEMPORARY BRACING SHALL BE INTRODUCED WHEREVER NECESSARY.
- FLOOR AND OVERHEAD STRUCTURE TO PROPERLY TRANSFER ALL IMPOSED LOADS TRACKS ABOVE STORE-FRONTS SHALL BE 14GA IN COMBINATION W/ STUD JOISTS

. TRACKS SHALL BE OF SAME GAUGE AND SHALL BE SECURELY ANCHORED TO THE

- 6. WHEN REQUIRED, INSTALLED FULL SIZE SHIMS BELOW BOTTOM TRACK AT STUD LOCATIONS AND/OR SET TRACK ON HIGH STRENGTH GROUT.
- 1. ERECT STUDS ONE PIECE FULL LENGTH, SPLICING IS NOT PERMITTED. 8. BRIDGING FOR WALL SHALL BE INSTALLED IN ACCORDANCE TO SCHEDULE SET

Langley Bypass

FORTH IN DRAWINGS AND/OR TO MANUFACTURERS RECOMMENDATIONS.

| 01 | 30/03/17 ISSUED FOR BP DATE DESCRIPTION

CONSULTANT



CONSULTANT SEAL

CONTRACTOR SHALL VERIFY ALL DIMENSIONS ON SITE. DRAWINGS SHALL NOT BE



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Langley Bypass Commercial

20670 LANGLEY BYPASS LANGLEY, BC

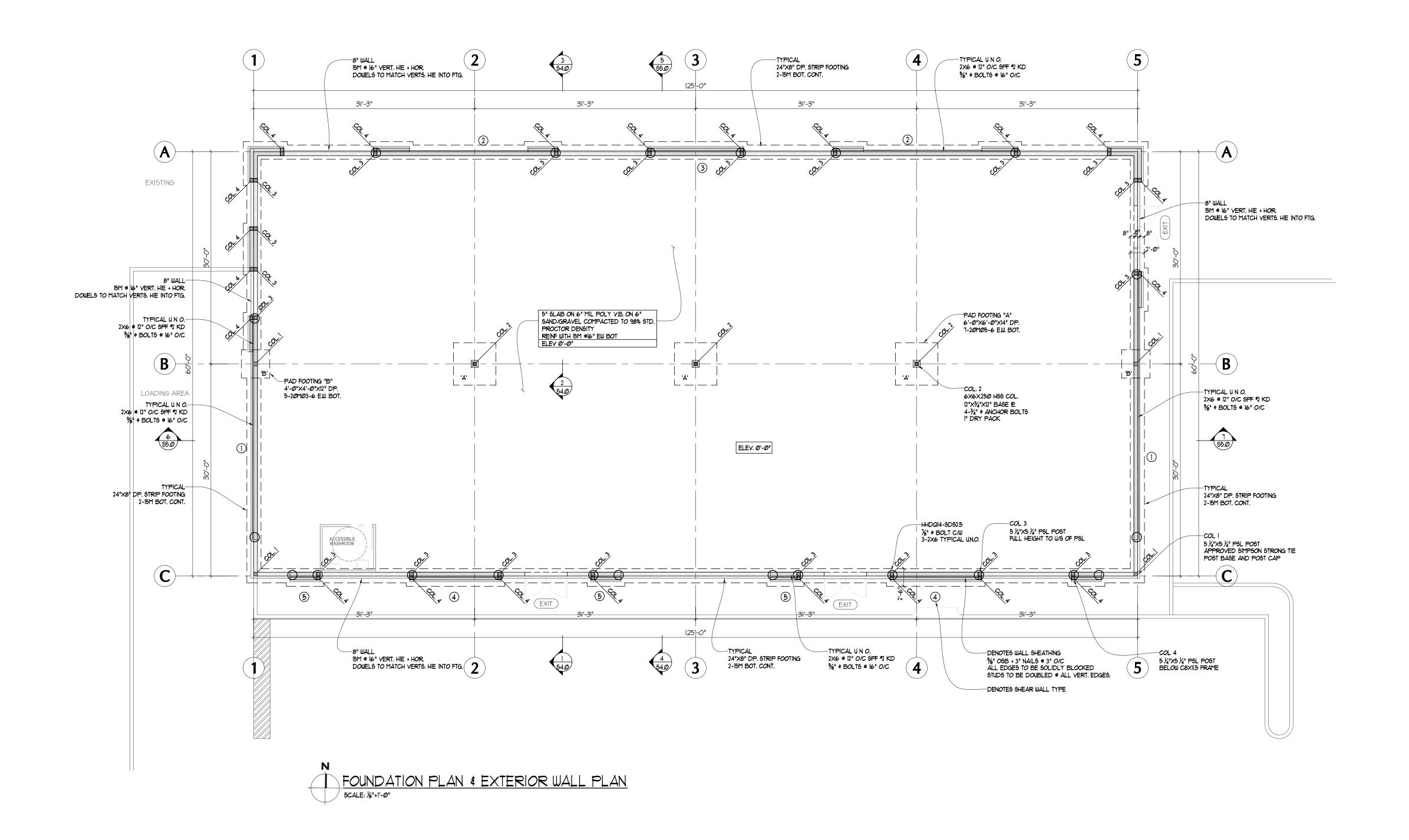
GENERAL NOTES & KEY PLAN

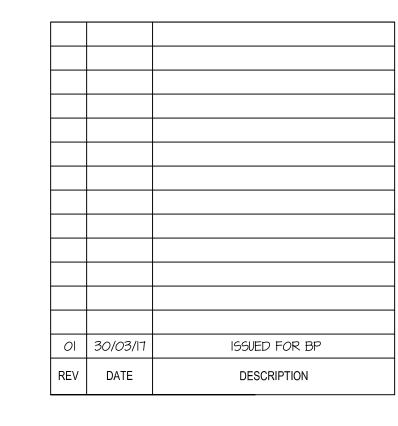
PROJECT NUMBER (13-94) DRAWING NUMBER S-1.0 PJL-263 SCALE

REVISION

DRAWING LIST: 5-1.0 GENERAL NOTES & KEY PLAN S-2.0 FOUNDATION PLAN S-2.Øa LOWER CANOPY PLAN ≰ DETAILS 6-3.0 ROOF FRAMING PLAN 5-40 SECTIONS 5-5.0 SECTIONS

LAZYBOY





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ARCI



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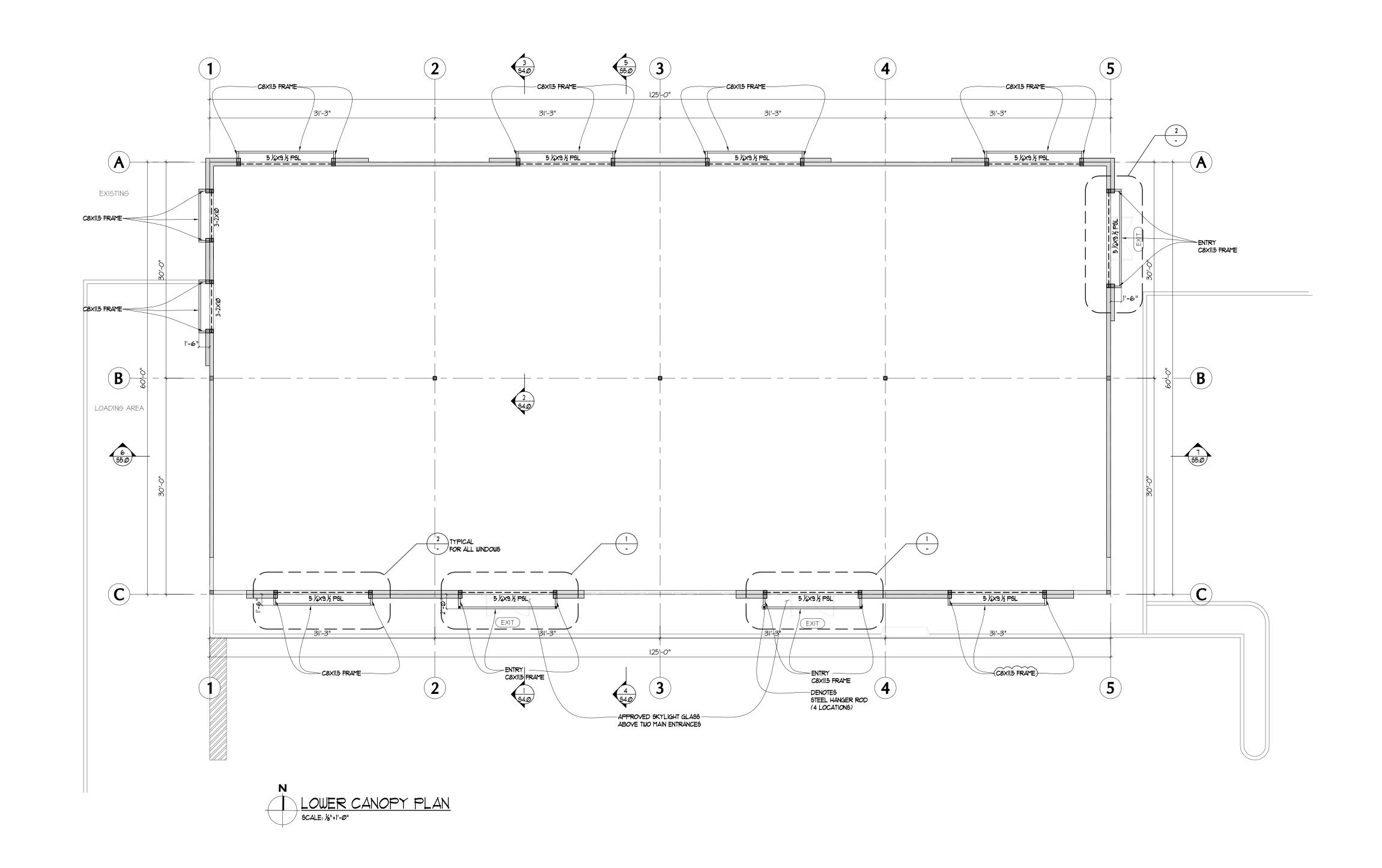
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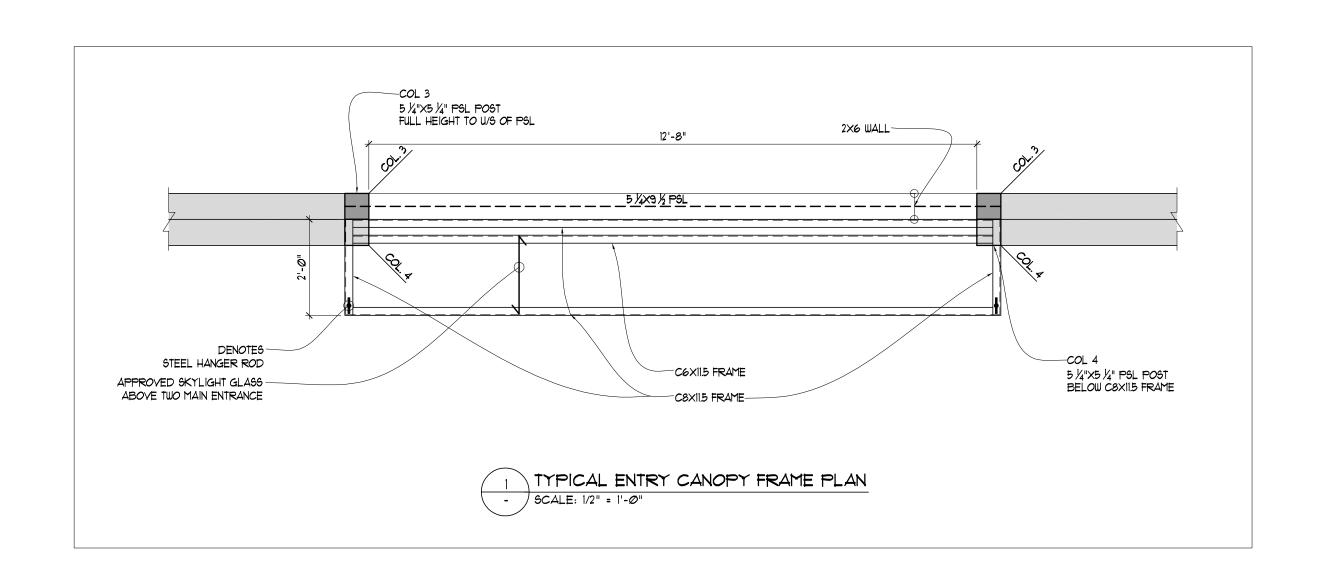
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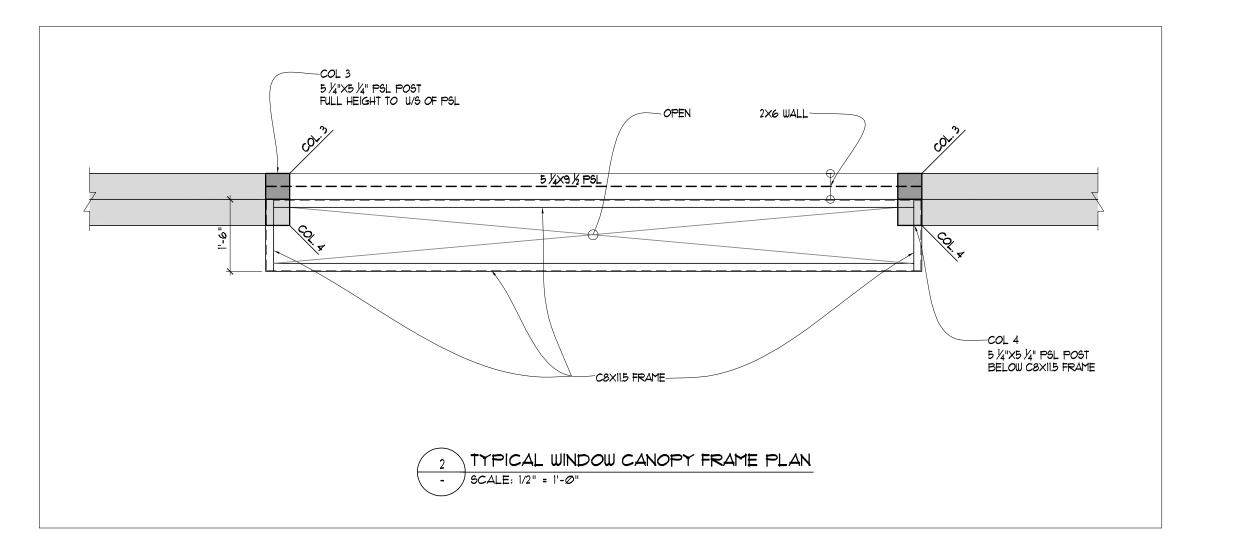
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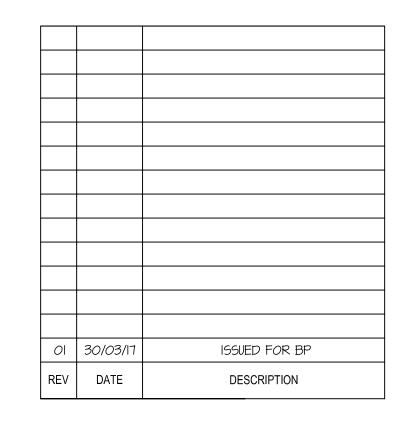
SCALE

FOUNDATION PLAN









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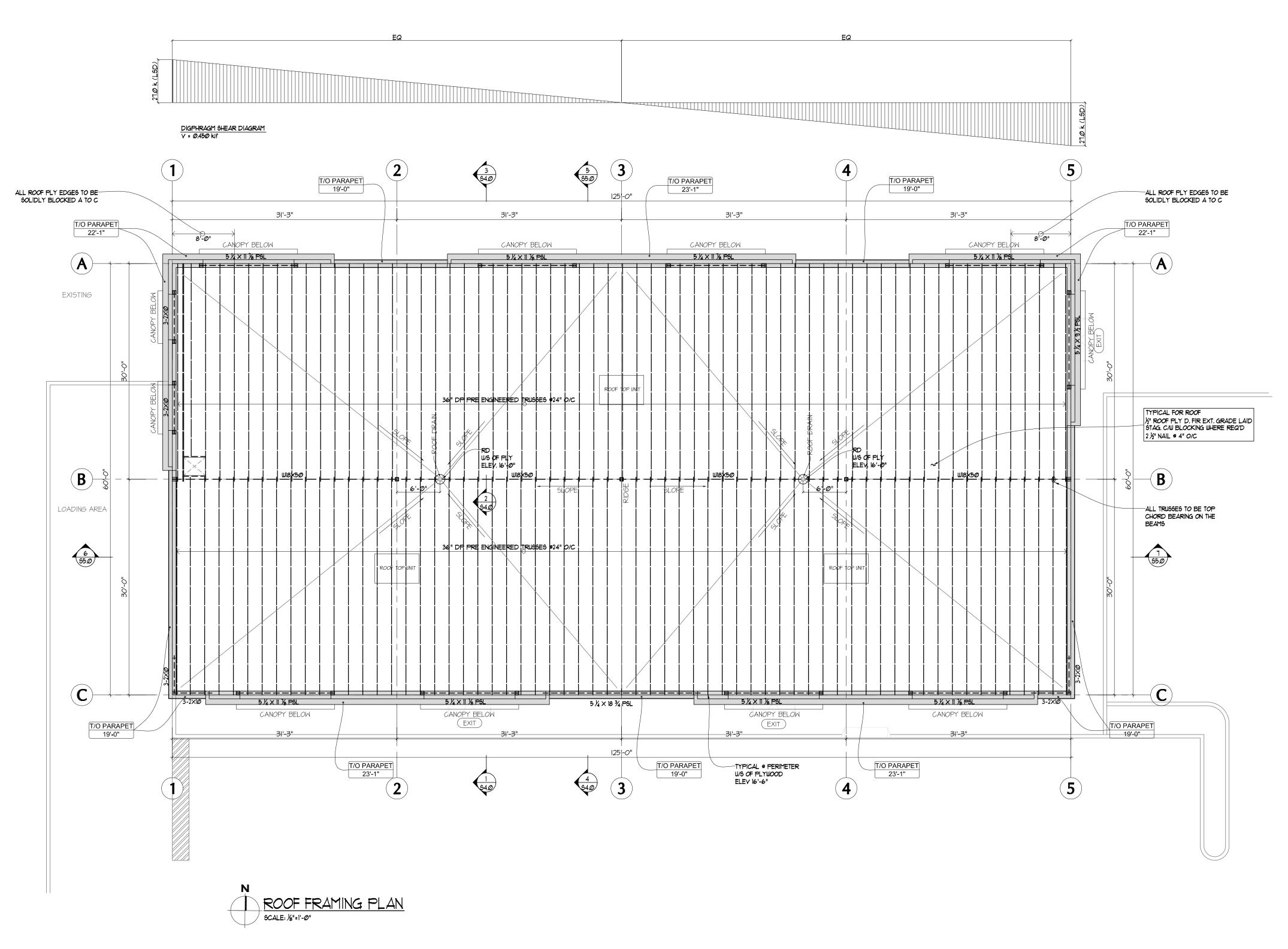
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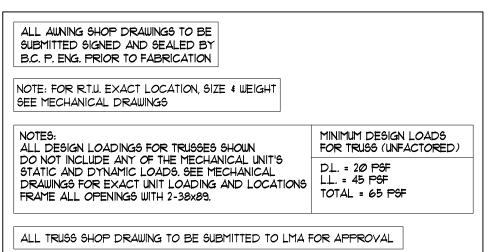
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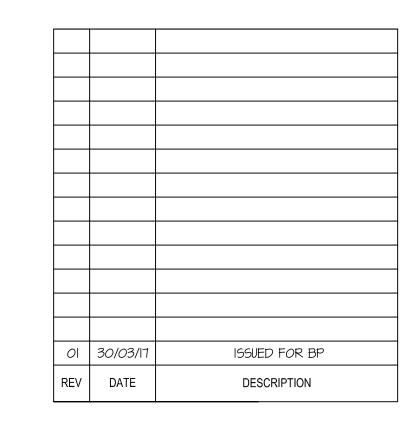
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LOWER CANOPY PLAN & DETAILS

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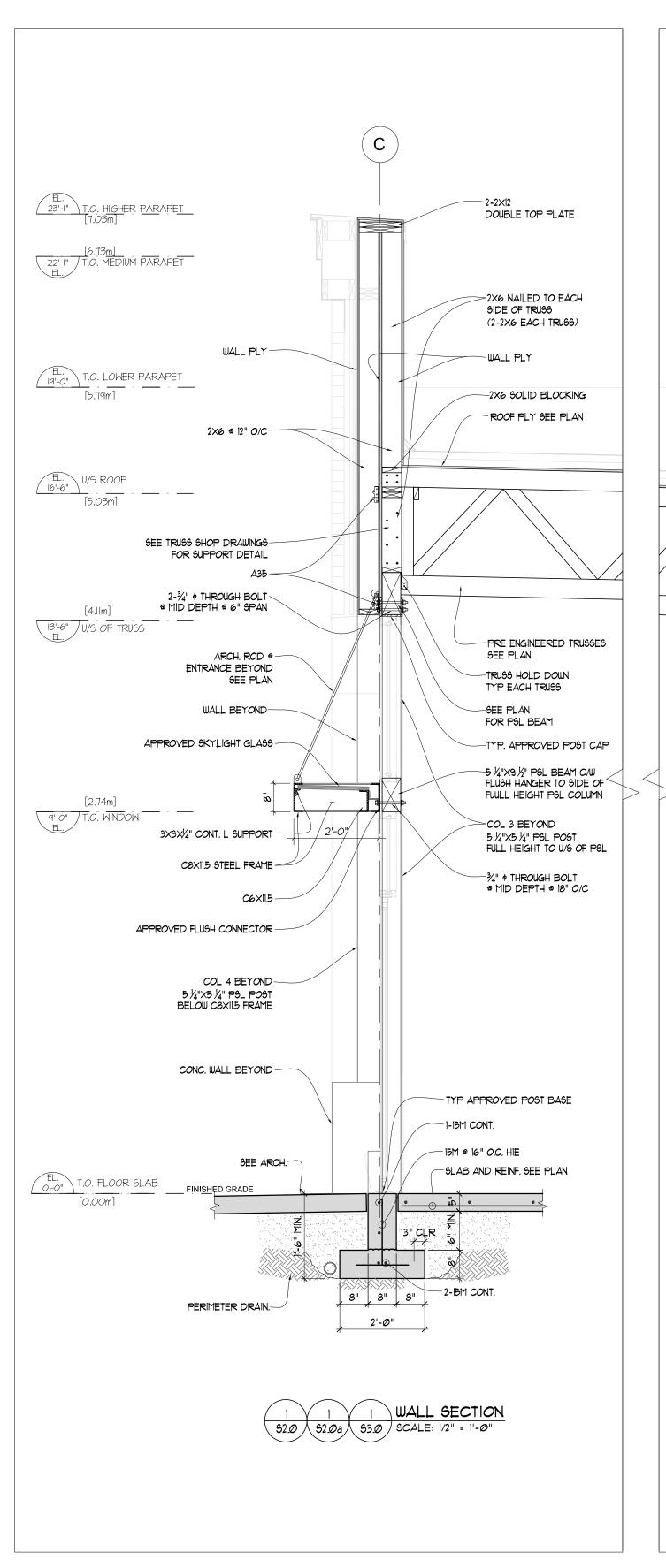
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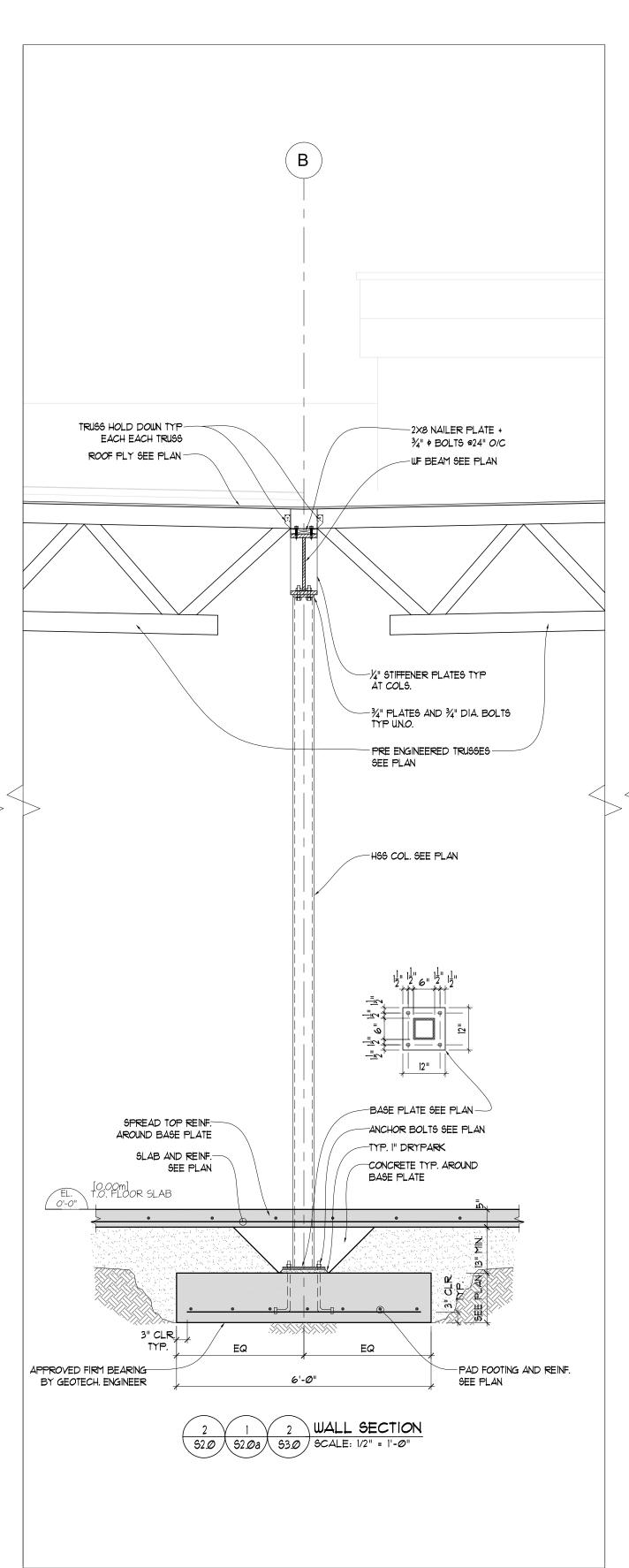
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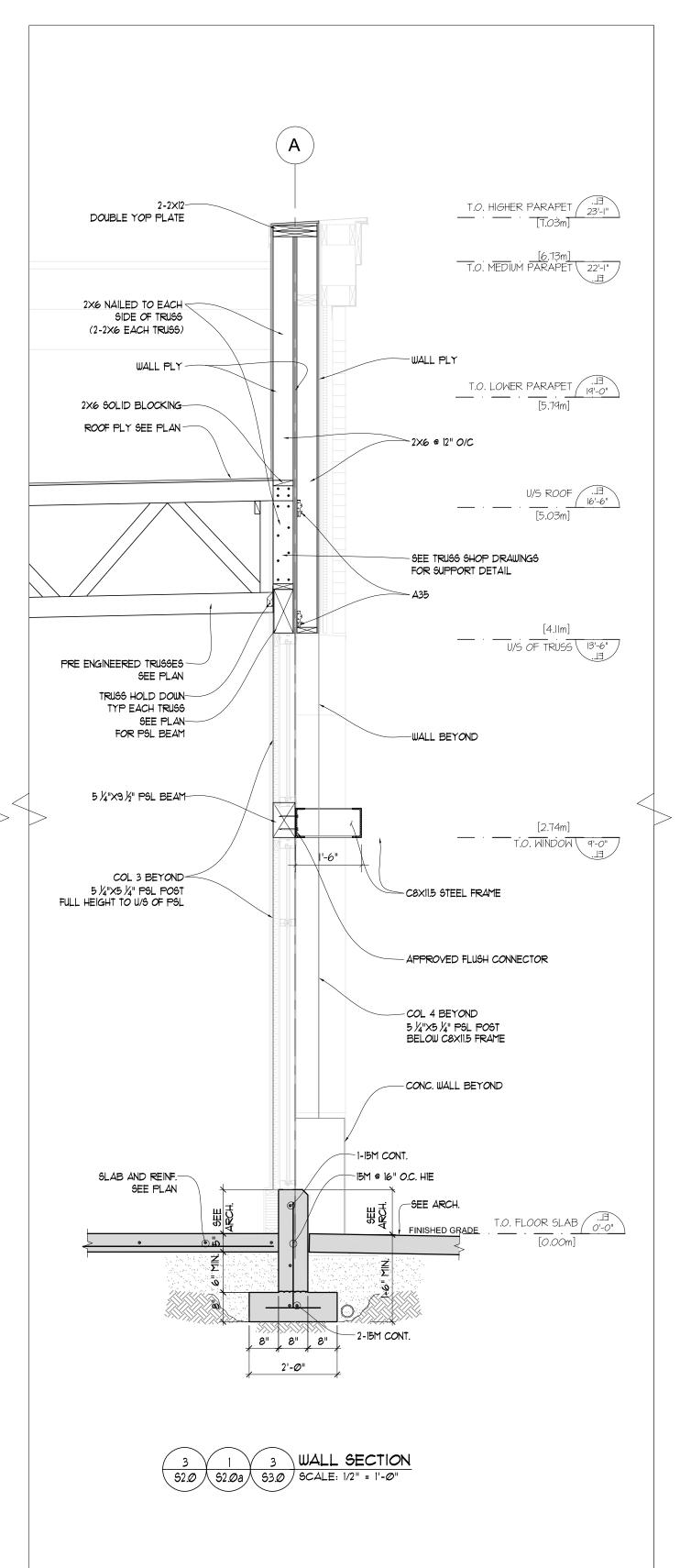
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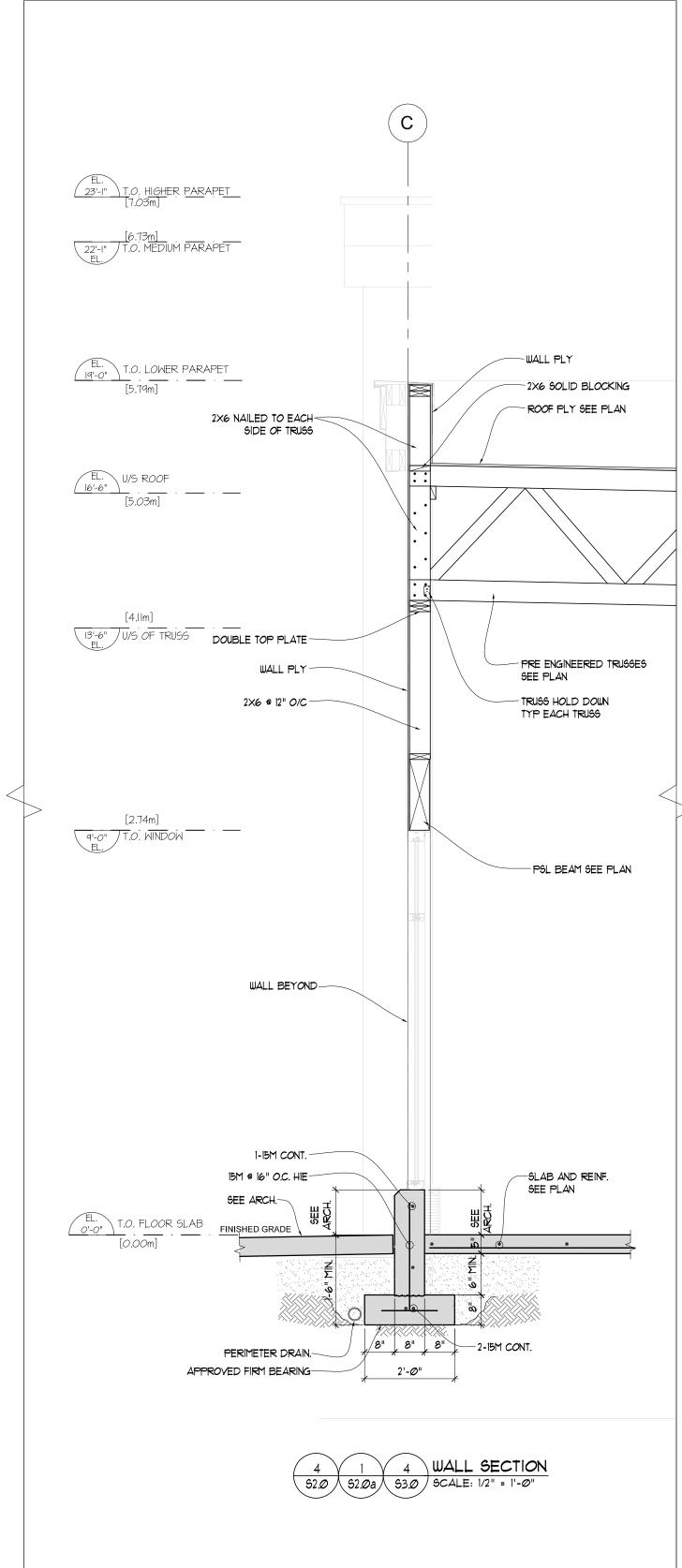
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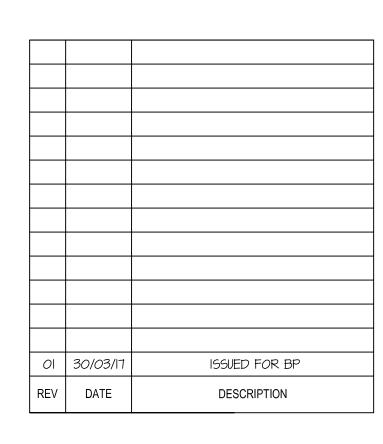
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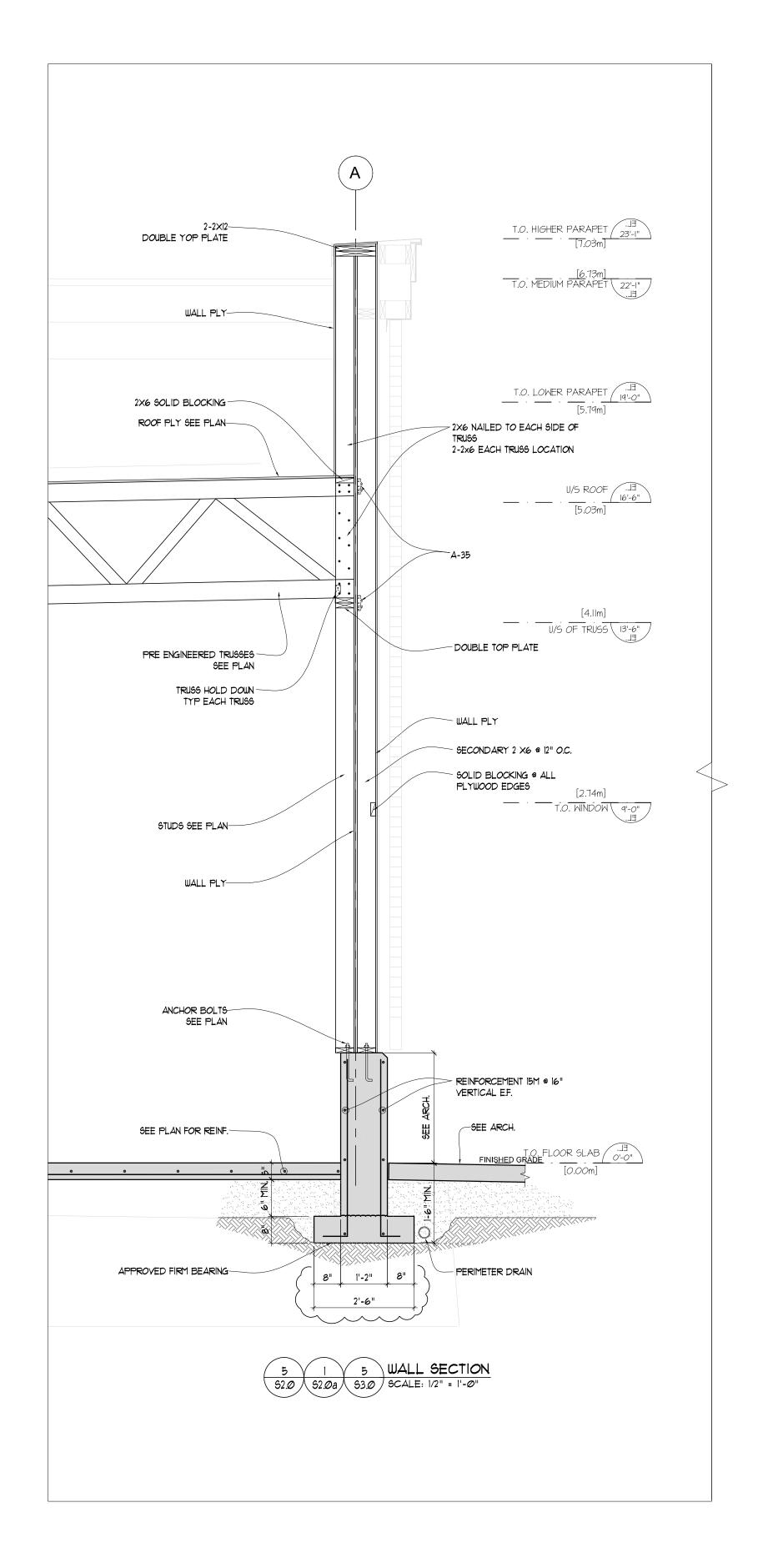
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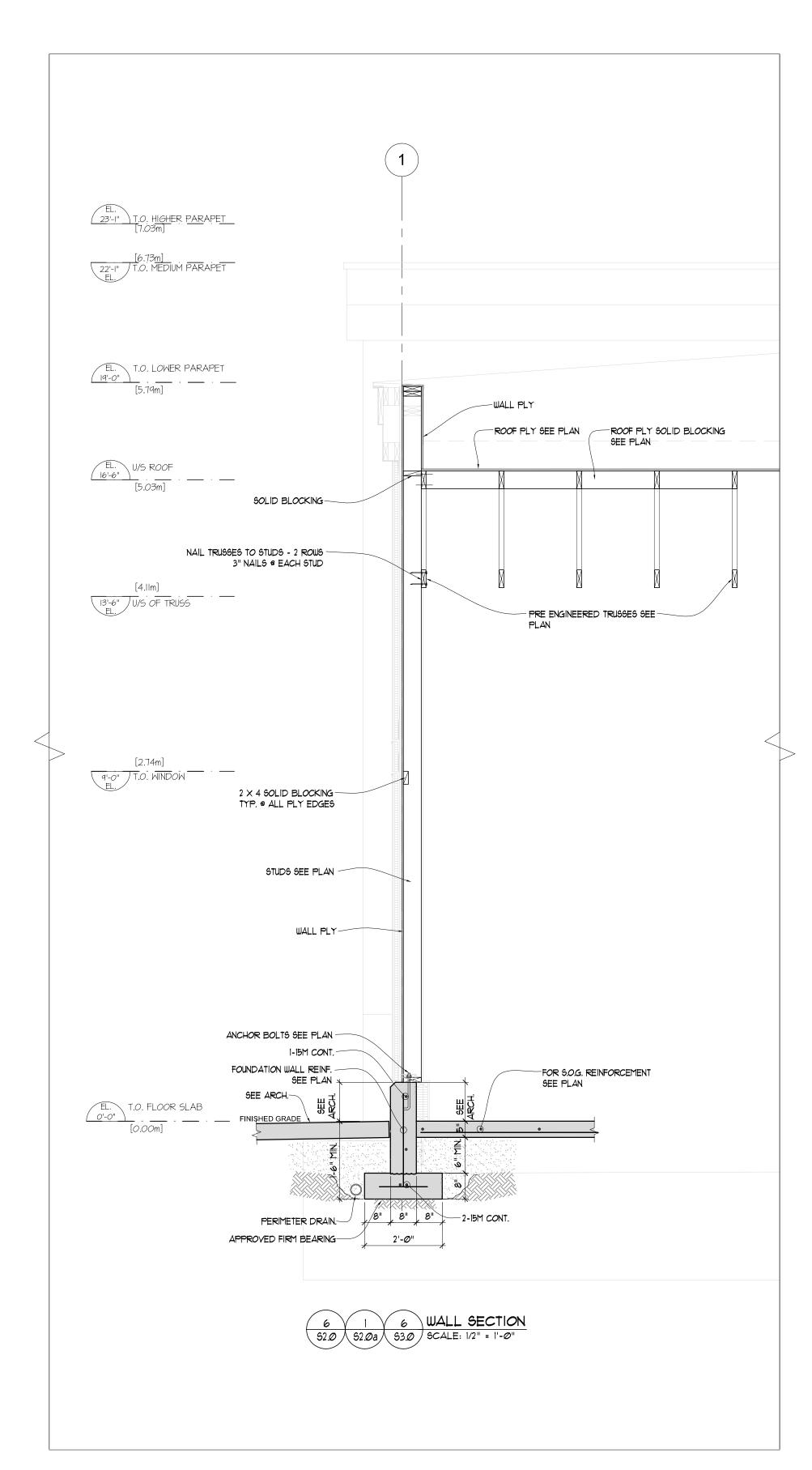
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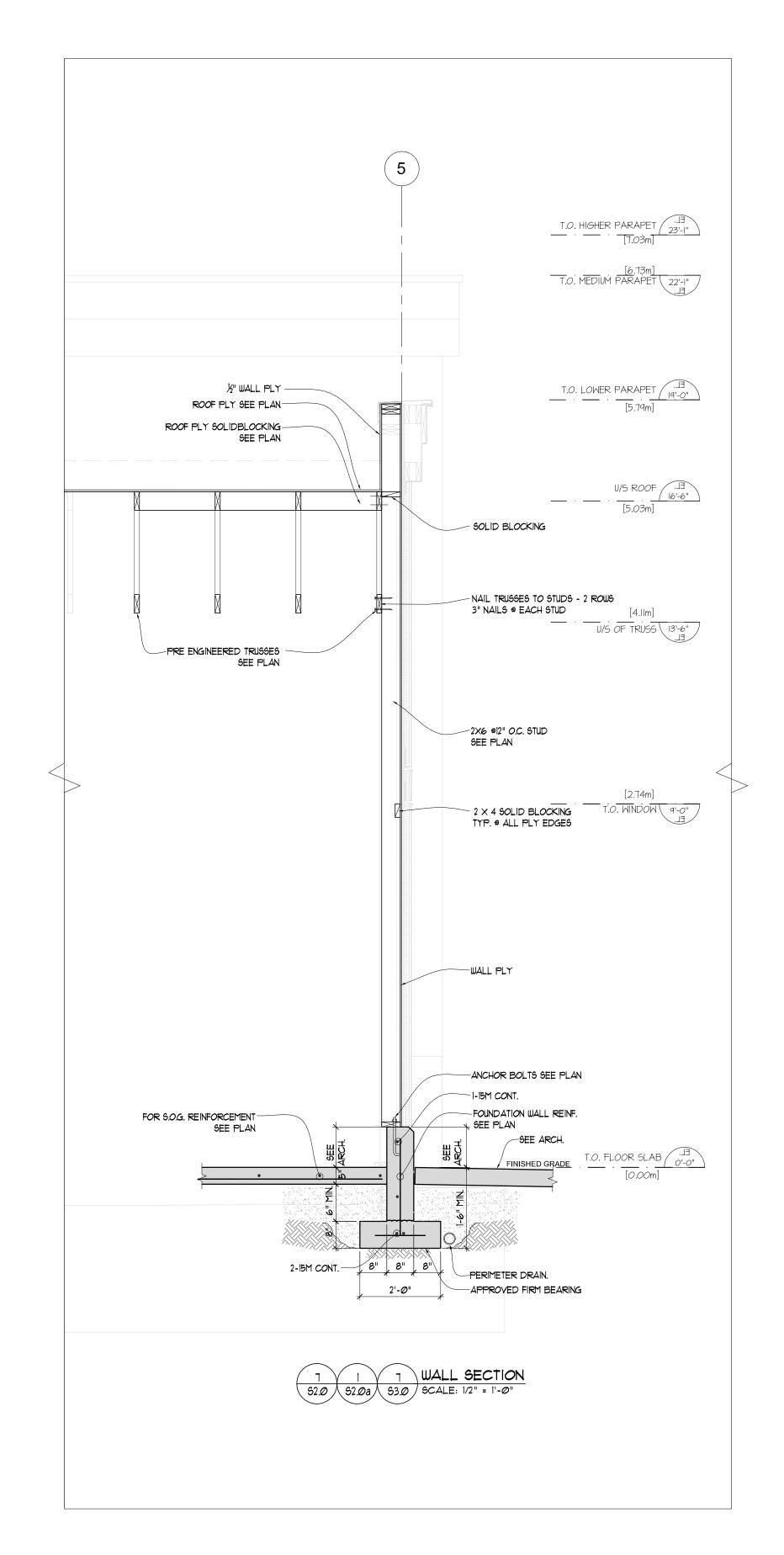
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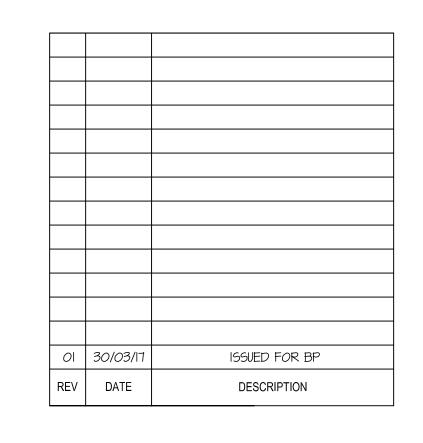
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